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CALCULATION PACKAGE

April 13, 2021

JayMarc Homes

6515 SE 30th St

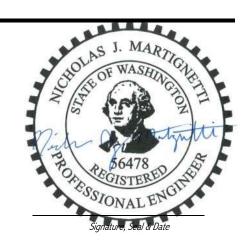
Mercer Island, Washington

MULHERN & KULP STRUCTURAL ENGINEERING, INC.

Prepared By:

Riley J. Denis, E.I.T. Staff Engineer

Nick J. Martignetti, P.E. Associate Owner + San Diego Office Director





6515 SE 30TH

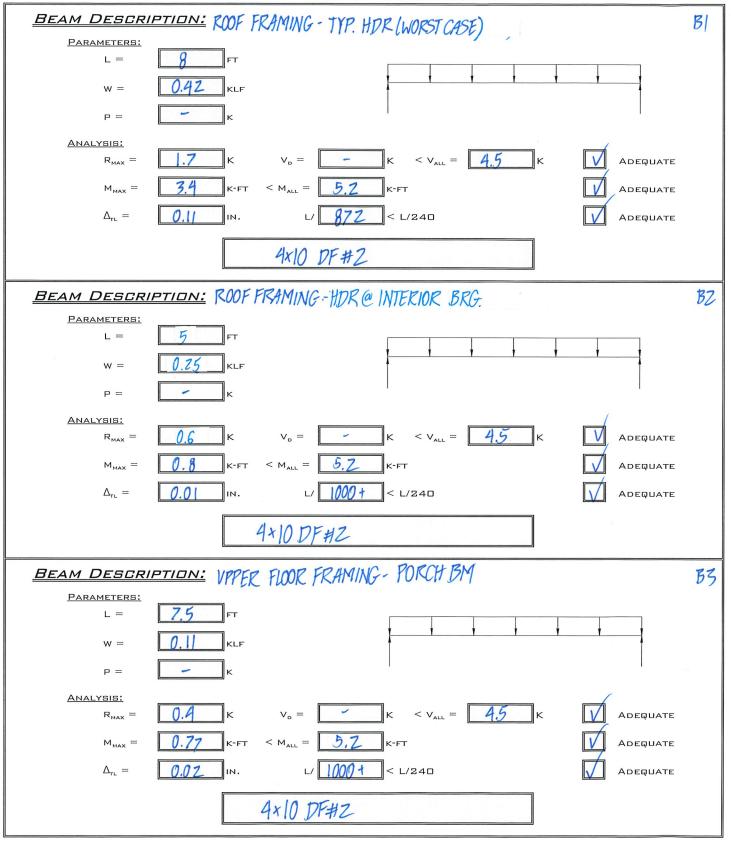
JAYMARC HOMES

M&K PROJECT #: 154-21007

ENGINEER:

RJD

DATE: 26-MAR-21





6515 SE 30TH

JAYMARC HOMES

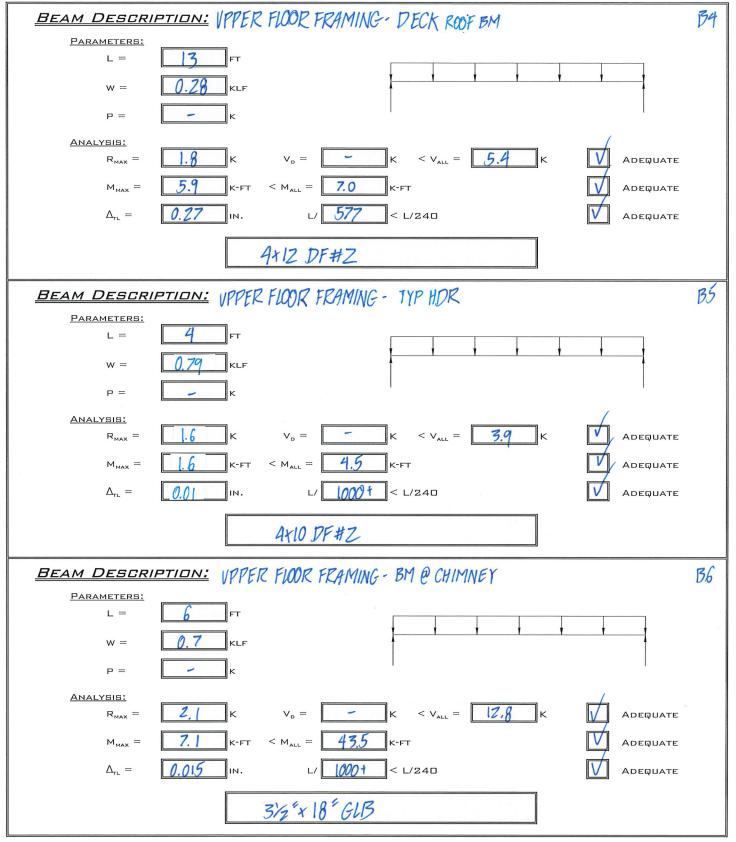
154-21007

M&K PROJECT #: ENGINEER:

DATE:

RJD

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6515 SE 30TH

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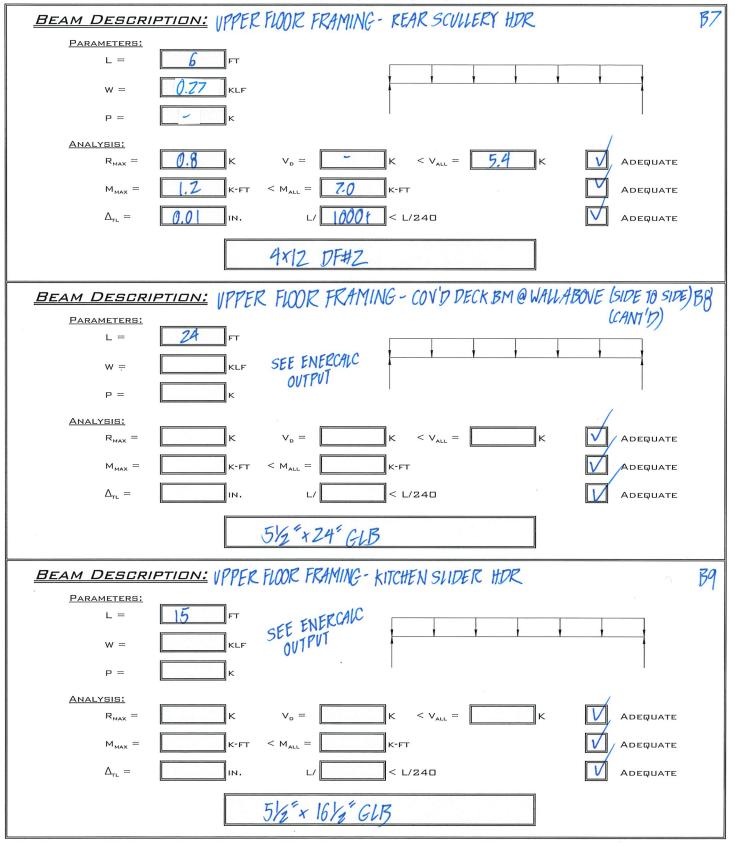
154-21007

M&K PROJECT #: ENGINEER:

DATE:

ER: RJD

26-Mar-21





PROJECT NAME: 6515 SE 30TH

JAYMARG HOMES

M&K PROJECT #: 154-21007

ENGINEER: RJD DATE: 26-MAR-21

BEAM DESCRIPTION: UPPER FLOOR FRAMING-BM@OWNERS SUITE WALL ABOVE	BIO
PARAMETERS:	
L = 17 FT SEE ENERCALC	
W = SEE ENERCALC OUTPUT	
P = K	
ANALYSIS: $R_{MAX} = $	
M _{MAX} = K-FT < M _{ALL} = K-FT ADEQUATE	
$\Delta_{ extsf{rL}} =$ $ extsf{In.}$ $ extsf{L}/ extsf{L}/ extsf{240}$ $ extsf{V}$ Adequate	
51/2 "x 18" GLB	
BEAM DESCRIPTION: UPPER FLOOR FRAMING - NOOK / HALL BM	BII
PARAMETERS:	
L = 12 FT $0.5k1F 10k1P$	
W =KLF	
P= - K 5 4' 4' 5	
ANALYSIS:	
$R_{MAX} = $	
$M_{MAX} = 13$ K-FT $M_{ALL} = 20.2$ K-FT	
$\Delta_{rL} = 0.35$ in. L/ 410 < L/240 \overline{V} Adequate	
5/2 * 10/2 " GLB	
BEAM DESCRIPTION: UPPER FLOOR FRAMING - REAR HALL HOR (PORTAL)	BIZ
PARAMETERS:	
L = 5 FT	
W = 0.13 KLF	
P = K	
Analysis:	
$R_{MAX} = 0.3$ K $V_D = $ K $< V_{ALL} = 4.7$ K ADEQUATE	
$M_{MAX} = $ $K-FT < M_{ALL} = $ $K-FT$ ADEQUATE	
$\Delta_{\text{TL}} = 0.005$ in. L/ $1000 + < \text{L/240}$ Adequate	
4x12 DF#Z	



6515 SE 30TH

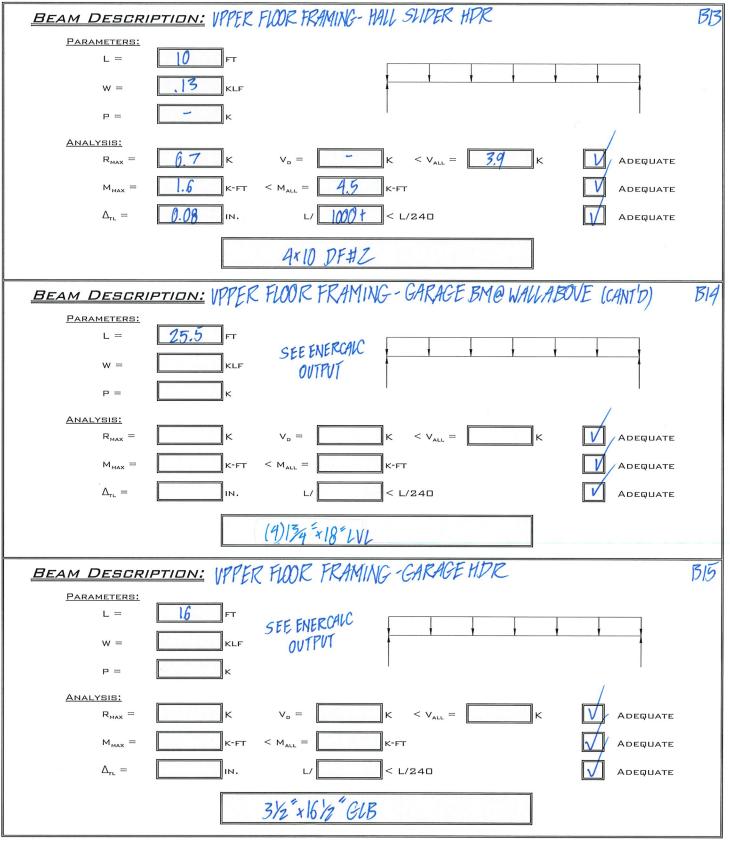
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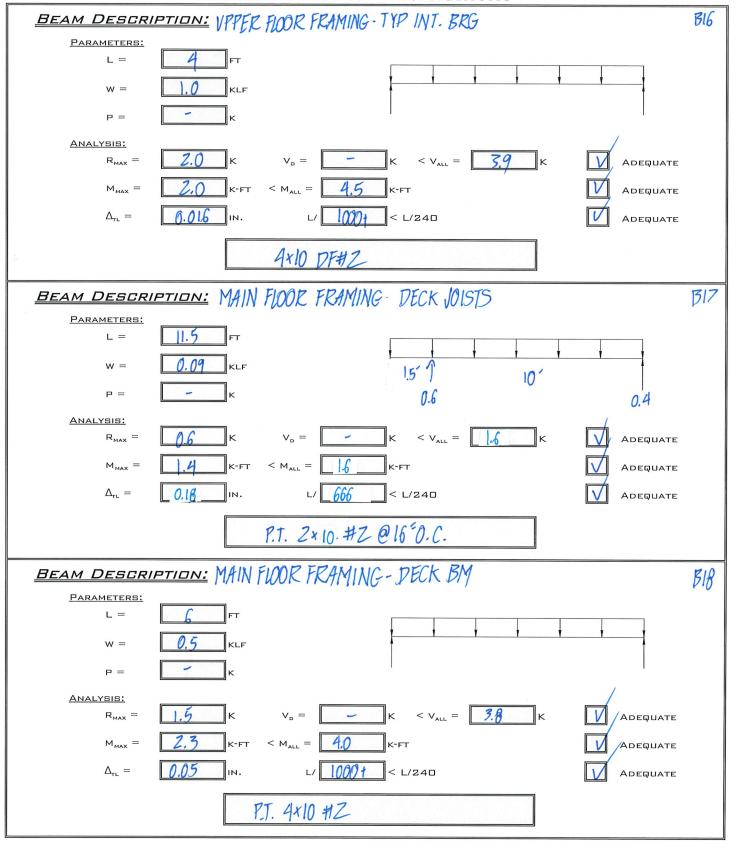
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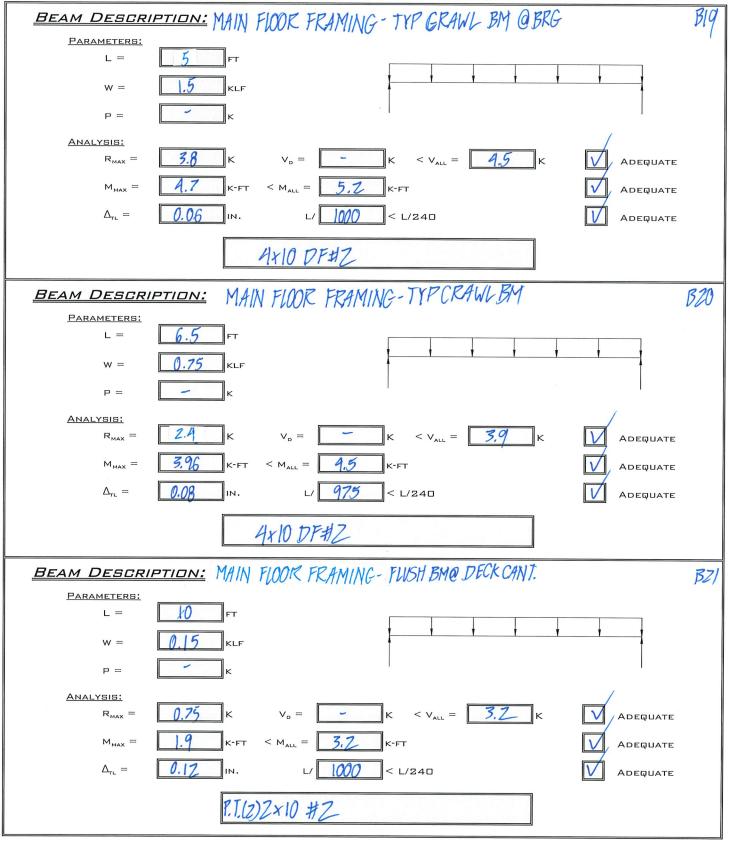
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PROJECT NAME: 6515 SE 30TH

JAYMARC HOMES

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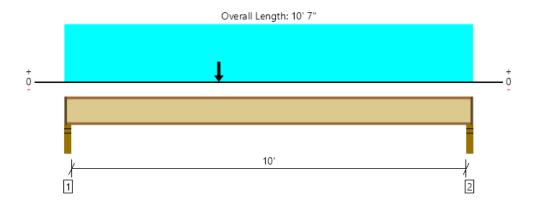
ENGINEER: RJD DATE: 26-MAR-21

BEAM DESCRIPTION: MAIN FLOOR FRAMING - DROPPED BM @ DECK CANT,	BZZ
PARAMETERS:	
L = 10 FT	
W = 0.28 KLF	
, b = K	
ANALYSIS: $R_{MAX} = $	
ADEQUA	TE
$M_{MAX} = 3.5$ K-FT $< M_{ALL} = 4.5$ K-FT	TE
$\Delta_{\text{TL}} = 0.19$ IN. L/ 630 < L/240 $\sqrt{}$ ADEQUA	TE
P.T. 4x10 #Z	
BEAM DESCRIPTION:	
PARAMETERS:	
L = FT	
W = KLF	
$P = $ κ	
Analysis:	
R _{MAX} = K V _D = K < V _{ALL} = K ADEQUAT	ΓE
M _{MAX} = K-FT < M _{ALL} = K-FT ADEQUA	ΓE
$\Delta_{rL} =$ IN. L/ $<$ L/240 Adequa	ΓE
BEAM DESCRIPTION:	
PARAMETERS:	
L = FT	,
W = KLF	
Ρ = κ	
Analysis:	
R _{MAX} = K V _D = K < V _{ALL} = K ADEQUAT	
M _{MAX} = K-FT < M _{ALL} = K-FT ADEQUAT	E
$\Delta_{rL} =$ IN. L/ $<$ L/240 ADEQUAT	E.



MEMBER REPORT

Level, Floor: Joist 1 piece(s) 11 7/8" TJI ® 210 @ 16" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1119 @ 2 1/2"	1134 (2.25")	Passed (99%)	1.00	1.0 D + 1.0 L (All Spans)
Shear (lbs)	1107 @ 3 1/2"	1655	Passed (67%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	3738 @ 4'	3795	Passed (98%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.145 @ 5' 7/8"	0.254	Passed (L/839)		1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.211 @ 5' 11/16"	0.508	Passed (L/578)		1.0 D + 1.0 L (All Spans)
TJ-Pro™ Rating	63	35	Passed		

System : Floor Member Type : Joist Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- · Allowed moment does not reflect the adjustment for the beam stability factor.
- A structural analysis of the deck has not been performed.
- Deflection analysis is based on composite action with a single layer of 23/32" Weyerhaeuser Edge™ Panel (24" Span Rating) that is glued and nailed down.
- Additional considerations for the TJ-Pro™ Rating include: None.

	Bearing Length			Loads t	o Supports		
Supports	Total	Available	Required	Dead	Floor Live	Total	Accessories
1 - Stud wall - SPF	3.50"	2.25"	2.19"	342	784	1126	1 1/4" Rim Board
2 - Stud wall - SPF	3.50"	2.25"	1.75"	232	581	813	1 1/4" Rim Board

[•] Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	3' 7" o/c	
Bottom Edge (Lu)	10' 5" o/c	

- •TJI joists are only analyzed using Maximum Allowable bracing solutions.
- $\bullet \mbox{Maximum allowable bracing intervals based on applied load.}$

			Dead	Floor Live	
Vertical Loads	Location (Side)	Spacing	(0.90)	(1.00)	Comments
1 - Uniform (PSF)	0 to 10' 7"	16"	10.0	40.0	Default Load
2 - Point (PLF)	4'	16"	325.0	600.0	

Weyerhaeuser Notes

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The product application, input design loads, dimensions and support information have been provided by ForteWEB Software Operator

ForteWEB Software Operator	Job Notes
Riley Denis Mulhern & Kulp Structural Engineers (215) 646-8001 rdenis@mulhernkulp.com	





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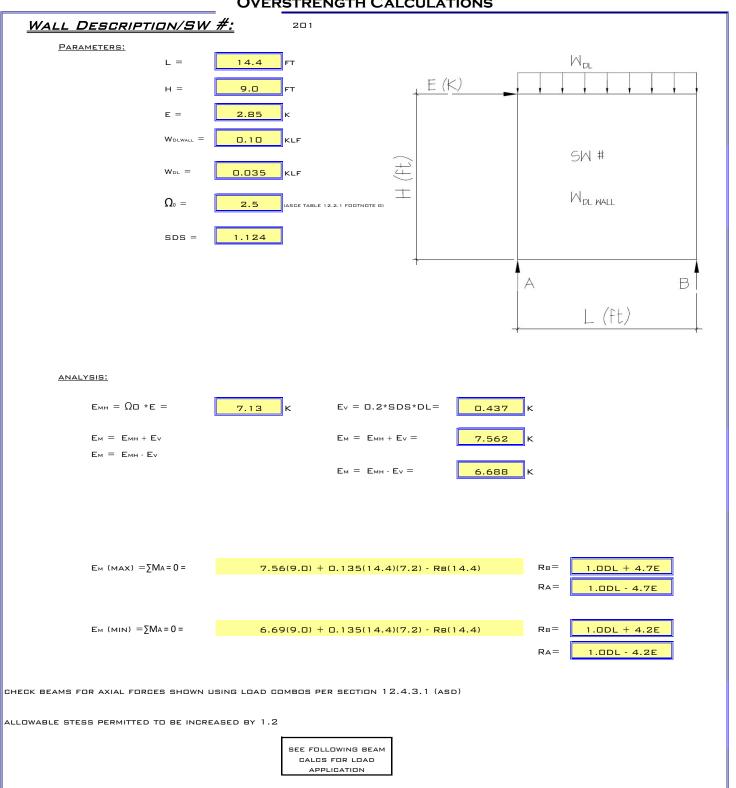
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> RJD ENGINEER: DATE: 09-Apr-21

OVERSTRENGTH CALCULATIONS





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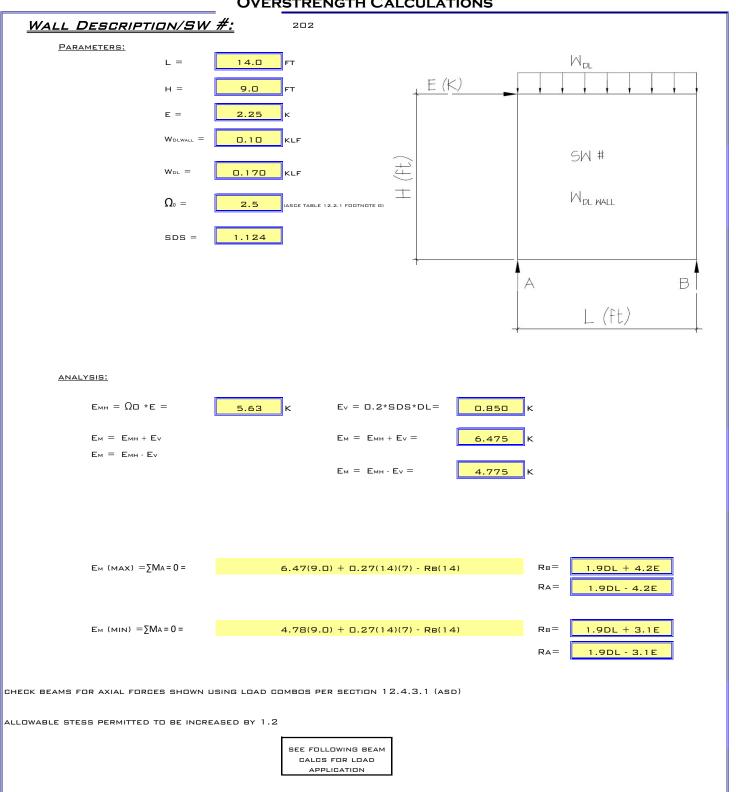
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OVERSTRENGTH CALCULATIONS





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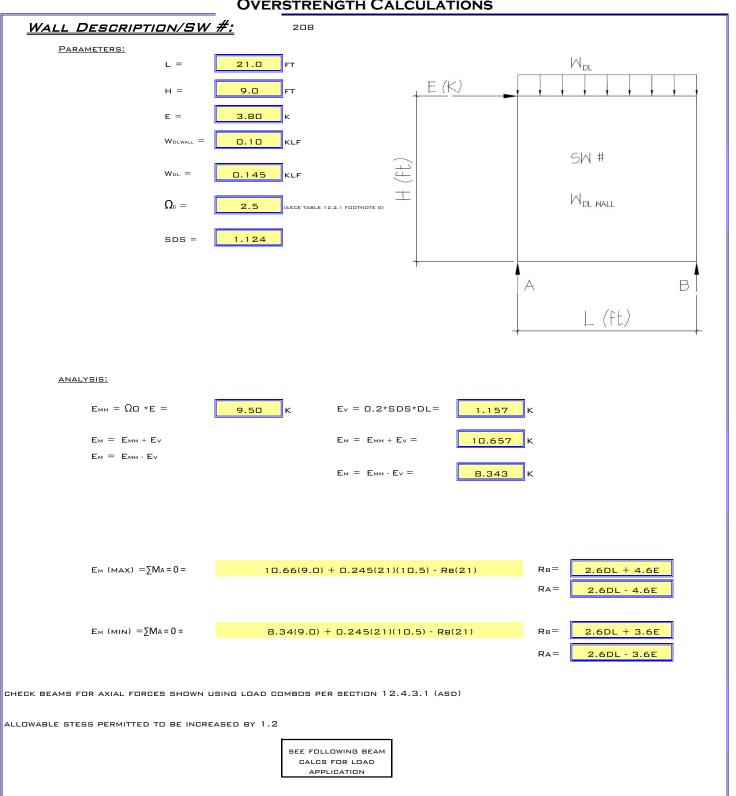
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154-21007 M&K PROJECT #:

> RJD ENGINEER: DATE: 09-Apr-21

OVERSTRENGTH CALCULATIONS



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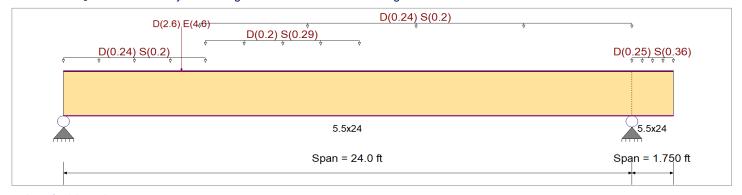
Lic. # : KW-06004787 **DESCRIPTION: BEAM B8**

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design	Fb+	2400 psi	E : Modulus of Elastic	ity
	Fb -	1850 psi	Ebend- xx	1800ksi
	Fc - Prll	1650 psi	Eminbend - xx	950ksi
Wood Species : DF/DF	Fc - Perp	650 psi	Ebend- yy	1600 ksi
Wood Grade : 24F-V4	Fv .	265 psi	Eminbend - yy	850ksi
	Ft	1100 psi	Density	31.21 pcf
Beam Bracing : Beam is Fully Braced against lateral-torsiona	l buckling	·	,	•



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loads

Load for Span Number 1

Uniform Load: D = 0.240, S = 0.20 k/ft, Extent = 0.0 -->> 6.0 ft, Tributary Width = 1.0 ft Uniform Load: D = 0.20, S = 0.290 k/ft, Extent = 6.0 -->> 12.50 ft, Tributary Width = 1.0 ft

Point Load: D = 2.60, E = 4.60 k @ 5.0 ft

Uniform Load: D = 0.240, S = 0.20 k/ft, Extent = 5.750 -->> 24.0 ft, Tributary Width = 1.0 ft

Load for Span Number 2

Uniform Load: D = 0.250, S = 0.360 k/ft, Extent = 0.0 -->> 1.750 ft, Tributary Width = 1.0 ft

DESIGN SUMMARY					Design OK
Maximum Bending Stress Ratio Section used for this span	=		ximum Shear Stress Ratio	=	0.328 : 1
·		5.5x24	Section used for this span		5.5x24
fb: Actual Fb: Allowable	=	1,274.21psi	fv: Actual Fv: Allowable	=	100.02 psi
	=	2,523.13psi		=	304.75 psi
Load Combination		+D+S	Load Combination		+D+S
Location of maximum on span	=	10.324ft	Location of maximum on span	=	0.000 ft
Span # where maximum occurs	=	Span # 1	Span # where maximum occurs	=	Span # 1
Maximum Deflection					
Max Downward Transient Deflec	tion	0.205 in Ratio =	1402 >= 360		
Max Upward Transient Deflection	n	-0.045 in Ratio =	932 >= 360		
Max Downward Total Deflection		0.540 in Ratio =	532 >= 300		
Max Upward Total Deflection		-0.116 in Ratio =	360 >= 300		

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+1.090D+0.750L+0.750S+0.5250E, L	1	0.5404	11.531		0.0000	0.000
	2	0.0000	11.531	+1.090D+0.750L+0.750S+0.5250E, L	-0.1160	1.750

Support notation : Far left is #1 **Vertical Reactions** Values in KIPS

			·
Load Combination	Support 1	Support 2	Support 3
Overall MAXimum	11.250	8.578	
Overall MINimum	3.642	0.958	
D Only	6.108	4.786	

Wood Beam Lic. # : KW-06004787

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DESCRIPTION: BEAM B8

Vertical Reactions Support notation : Far left is #1 Values in KIPS

vertical Reactions		Sup	port notation : Far left is # i	values in KIPS	
Load Combination	Support 1	Support 2	Support 3		
+D+L, LL Comb Run (*L)	6.108	4.786			
+D+L, LL Comb Run (L*)	6.108	4.786			
+D+L, LL Comb Run (LL)	6.108	4.786			
+D+Lr, LL Comb Run (*L)	6.108	4.786			
+D+Lr, LL Comb Run (L*)	6.108	4.786			
+D+Lr, LL Comb Run (LL)	6.108	4.786			
+D+S	9.681	8.578			
+D+0.750Lr+0.750L, LL Comb Run (*L)	6.108	4.786			
+D+0.750Lr+0.750L, LL Comb Run (L*)	6.108	4.786			
+D+0.750Lr+0.750L, LL Comb Run (LL)	6.108	4.786			
+D+0.750L+0.750S, LL Comb Run (*L)	8.788	7.630			
+D+0.750L+0.750S, LL Comb Run (L*)	8.788	7.630			
+D+0.750L+0.750S, LL Comb Run (LL)	8.788	7.630			
+D+0.60W	6.108	4.786			
+1.126D+0.70E	9.427	6.060			
+D+0.750Lr+0.750L+0.450W, LL Comb R	6.108	4.786			
+D+0.750Lr+0.750L+0.450W, LL Comb R	6.108	4.786			
+D+0.750Lr+0.750L+0.450W, LL Comb R	6.108	4.786			
+D+0.750L+0.750S+0.450W, LL Comb Ru	8.788	7.630			
+D+0.750L+0.750S+0.450W, LL Comb Ru	8.788	7.630			
+D+0.750L+0.750S+0.450W, LL Comb Ru	8.788	7.630			
+1.090D+0.750L+0.750S+0.5250E, LL C	11.250	8.564			
+1.090D+0.750L+0.750S+0.5250E, LL C	11.250	8.564			
+1.090D+0.750L+0.750S+0.5250E, LL C	11.250	8.564			
+0.60D+0.60W	3.665	2.872			
+0.470D+0.70E	5.420	2.920			
D Only	6.108	4.786			
S Only	3.573	3.792			
E Only	3.642	0.958			
H Only					

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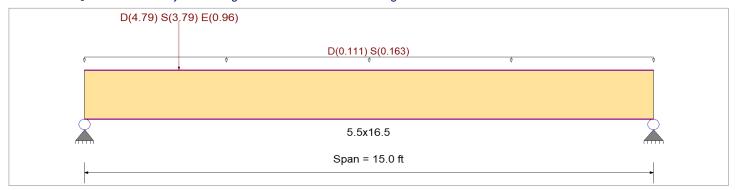
DESCRIPTION: BEAM B9

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design	Fb+	2400 psi	E : Modulus of Elastici	ty		
	Fb -	1850 psi	Ebend- xx	1800 ksi		
	Fc - Prll	1650 psi	Eminbend - xx	950 ksi		
Wood Species : DF/DF	Fc - Perp	650 psi	Ebend- yy	1600 ksi		
Wood Grade : 24F-V4	Fv .	265 psi	Eminbend - yy	850 ksi		
11000 0.000	Ft	1100 psi	Density	31.21 pcf		
Beam Bracing : Beam is Fully Braced against lateral-torsional buckling						



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loads

Point Load : D = 4.790, S = 3.790, E = 0.960 k @ 2.50 ft, (FROM B8) Uniform Load : D = 0.1110, S = 0.1630, Tributary Width = 1.0 ft

DESIGN SUMMARY					Design OK
Maximum Bending Stress Ratio	=	0.393 : 1 Ma	ximum Shear Stress Ratio	=	0.485 : 1
Section used for this span		5.5x16.5	Section used for this span		5.5x16.5
fb: Actual	=	1,080.26psi	fv: Actual	=	147.94 psi
Fb: Allowable	=	2,745.52psi	Fv: Allowable	=	304.75 psi
Load Combination		+D+S	Load Combination		+D+S
Location of maximum on span	=	2.628ft	Location of maximum on span	=	0.000 ft
Span # where maximum occurs	=	Span # 1	Span # where maximum occurs	=	Span # 1
Maximum Deflection					
Max Downward Transient Defle	ction	0.111 in Ratio =	1616 >= 360		
Max Upward Transient Deflection	on	0.000 in Ratio =	<mark>0</mark> < 360		
Max Downward Total Deflection		0.229 in Ratio =	786 >= 300		
Max Upward Total Deflection		0.000 in Ratio =	<mark>0</mark> <300		

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+D+S	1	0.2290	6.898		0.0000	0.000
Vertical Reactions			Suppo	rt notation : Far left is #1	Values in KIPS	
Load Combination		Suppor	t 1 Support 2			
Overall MAXimum		9.3	3.633			
Overall MINimum		0.0	0.160			
D Only		4.9	72 1.778			
+D+L		4.9	772 1.778			
+D+Lr		4.9	72 1.778			
+D+S		9.3	3.633			
+D+0.750Lr+0.750L		4.9	772 1.778			
+D+0.750L+0.750S		8.2	257 3.169			
+D+0.60W		4.9	72 1.778			

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Wood Beam

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Verti	cal	Reac	tions

Support notation: Far left is #1

Values in KIPS

Vertical incactions		очрр	ort flotation . Fair lort is # i	Values III KII S	
Load Combination	Support 1	Support 2			
+1.126D+0.70E	6.158	2.114			
+D+0.750Lr+0.750L+0.450W	4.972	1.778			
+D+0.750L+0.750S+0.450W	8.257	3.169			
+1.090D+0.750L+0.750S+0.5250E	9.125	3.413			
+0.60D+0.60W	2.983	1.067			
+0.470D+0.70E	2.897	0.948			
D Only	4.972	1.778			
S Only	4.381	1.854			
E Only	0.800	0.160			
H Only					

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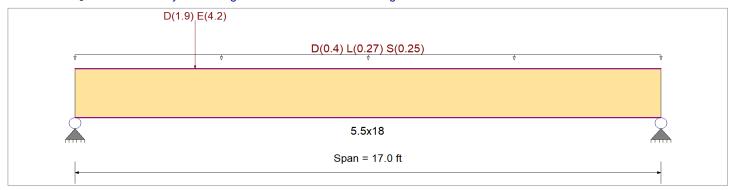
DESCRIPTION: BEAM B10

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Material Properties

Analysis Method: Allowable Stress Design	Fb+	2,400.0 psi	E : Modulus of Elasti	icity	
	Fb -	1,850.0 psi	Ebend- xx	1,800.0ksi	
	Fc - Prll	1,650.0 psi	Eminbend - xx	950.0ksi	
Wood Species : DF/DF	Fc - Perp	650.0 psi	Ebend- yy	1,600.0ksi	
Wood Grade : 24F-V4	Fv .	265 .0 psi	Eminbend - yy	850.0ksi	
	Ft	1,100.0 psi	Density	31.210 pcf	
Beam Bracing : Beam is Fully Braced against lateral-torsional buckling					



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loads Point Load : D = 1.90, E = 4.20 k @ 3.50 ft

Uniform Load : D = 0.40, L = 0.270, S = 0.250, Tributary Width = 1.0 ft

DESIGN SUMMARY					Design OK
Maximum Bending Stress Ratio Section used for this span fb: Actual	=	0.492 1 Ma 5.5x18 1,322.55psi	ximum Shear Stress Ratio Section used for this span fy: Actual	=	0.363 : 1 5.5x18 96.31 psi
Fb: Allowable	=	2,687.88psi	Fv: Allowable	=	265.00 psi
Load Combination Location of maximum on span Span # where maximum occurs	= =	+D+0.750L+0.750S 8.004ft Span # 1	Load Combination Location of maximum on span Span # where maximum occurs	= =	+D+L 0.000 ft Span # 1
Maximum Deflection Max Downward Transient Deflection Max Upward Transient Deflection Max Downward Total Deflection Max Upward Total Deflection	n	0.106 in Ratio = 0.000 in Ratio = 0.426 in Ratio = 0.000 in Ratio =	1923 >= 360 0 < 360 478 >= 300 0 < 300		

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+1.090D+0.750L+0.750S+0.5250E	1	0.4263	8.314		0.0000	0.000
Vertical Reactions			Suppo	rt notation : Far left is #1	Values in KIPS	
Load Combination		Suppor	1 Support 2			
Overall MAXimum		10.6	15 8.100			
Overall MINimum		3.3	35 0.865			
D Only		5.0	91 3.974			
+D+L		7.3	86 6.269			
+D+Lr		5.0	91 3.974			
+D+S		7.2	16 6.099			
+D+0.750Lr+0.750L		6.8	12 5.695			
+D+0.750L+0.750S		8.4	06 7.289			
+D+0.60W		5.0	91 3.974			

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Wood Beam Lic. # : KW-06004787

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DESCRIPTION: BEAM B10

Vertical Reactions Support notation: Far left is #1 Values in KIPS

VOI LIGAT TEGACTIONS			14.400
Load Combination	Support 1	Support 2	
+1.126D+0.70E	8.067	5.080	
+D+0.750Lr+0.750L+0.450W	6.812	5.695	
+D+0.750L+0.750S+0.450W	8.406	7.289	
+1.090D+0.750L+0.750S+0.5250E	10.615	8.100	
+0.60D+0.60W	3.055	2.384	
+0.470D+0.70E	4.728	2.473	
D Only	5.091	3.974	
L Only	2.295	2.295	
S Only	2.125	2.125	
E Only	3.335	0.865	
H Only			
-			

File: beam calcs with overstrength.ec6

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Lic. #: KW-06004787

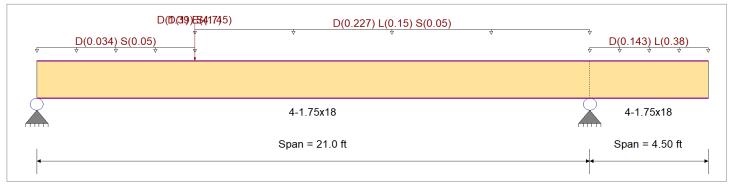
DESCRIPTION: BEAM B14

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design	Fb+	2600 psi	E : Modulus of Elasticity					
	Fb -	2600 psi	Ebend- xx	2000ksi				
	Fc - Prll	2510 psi	Eminbend - xx	1016.535 ksi				
Wood Species : iLevel Truss Joist	Fc - Perp	750 psi						
Wood Grade : MicroLam LVL 2.0 E	Fv .	285 psi						
	Ft	1555 psi	Density	42.01 pcf				
Beam Bracing : Beam is Fully Braced against lateral-torsional	Beam is Fully Braced against lateral-torsional buckling							



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loads

Load for Span Number 1

Point Load: D = 1.0, E = 4.70 k @ 6.0 ft

Uniform Load: D = 0.0340, S = 0.050 k/ft, Extent = 0.0 -->> 6.0 ft, Tributary Width = 1.0 ft

Uniform Load: D = 0.2270, L = 0.150, S = 0.050 k/ft, Extent = 6.0 -->> 21.0 ft, Tributary Width = 1.0 ft

Point Load : D = 1.390, S = 1.450 k @ 6.0 ft

Load for Span Number 2

Uniform Load : D = 0.1430, L = 0.380, Tributary Width = 1.0 ft

DESIGN SUMMARY					Design OK
Maximum Bending Stress Ratio Section used for this span	=	0.323 1 4-1.75x18	Maximum Shear Stress Ration Section used for this sp		0.185 : 1 4-1.75x18
fb: Actual	=	964.77 psi	fv: Actual	=	84.26 psi
Fb: Allowable	=	2,990.00psi	Fv: Allowable	=	456.00 psi
Load Combination +D+0.750	L+0.750S, LI	Comb Run (L*)	Load Combination	+1.090D+0.750L+0.	750S+0.5250E, LL
Location of maximum on span	=	8.564ft	Location of maximum on spa	n =	0.000 ft
Span # where maximum occurs	=	Span # 1	Span # where maximum occ	urs =	Span # 1
Maximum Deflection					
Max Downward Transient Deflect	ion	0.180 in Ratio	= 1399>=360		
Max Upward Transient Deflection	I	-0.104 in Ratio	= 1042 >= 360		
Max Downward Total Deflection		0.465 in Ratio	= 541 >= 300		
Max Upward Total Deflection		-0.288 in Ratio			

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Span	Load Combination	Max. "+" Defl	Location in Span
+1.090D+0.750L+0.750S+0.5250E, L	1	0.4651	10.089		0.0000	0.000
	2	0.0000	10.089	+1.090D+0.750L+0.750S+0.5250E, L	-0.2880	4.500
Vertical Reactions			Support	Values in KIPS		

Vertical ineactions		Oup	port notation : r ai loit is # i	Values III KII S	
Load Combination	Support 1	Support 2	Support 3		
Overall MAXimum	7.239	8.473			
Overall MINimum	3.357	1.343			
D Only	3.397	4.182			

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Lic. # : KW-06004787 DESCRIPTION: BEAM B14

Vertical Reactions		Sup	pport notation : Far left is #1	Values in KIPS
Load Combination	Support 1	Support 2	Support 3	
+D+L, LL Comb Run (*L)	3.214	6.076		
+D+L, LL Comb Run (L*)	4.201	5.629		
+D+L, LL Comb Run (LL)	4.018	7.522		
+D+Lr, LL Comb Run (*L)	3.397	4.182		
+D+Lr, LL Comb Run (L*)	3.397	4.182		
+D+Lr, LL Comb Run (LL)	3.397	4.182		
+D+S	4.958	5.122		
+D+0.750Lr+0.750L, LL Comb Run (*L)	3.260	5.602		
+D+0.750Lr+0.750L, LL Comb Run (L*)	4.000	5.267		
+D+0.750Lr+0.750L, LL Comb Run (LL)	3.863	6.687		
+D+0.750L+0.750S, LL Comb Run (*L)	4.430	6.307		
+D+0.750L+0.750S, LL Comb Run (L*)	5.171	5.972		
+D+0.750L+0.750S, LL Comb Run (LL)	5.033	7.392		
+D+0.60W	3.397	4.182		
+1.126D+0.70E	6.175	5.649		
+D+0.750Lr+0.750L+0.450W, LL Comb R	3.260	5.602		
+D+0.750Lr+0.750L+0.450W, LL Comb R	4.000	5.267		
+D+0.750Lr+0.750L+0.450W, LL Comb R	3.863	6.687		
+D+0.750L+0.750S+0.450W, LL Comb Ru	4.430	6.307		
+D+0.750L+0.750S+0.450W, LL Comb Ru	5.171	5.972		
+D+0.750L+0.750S+0.450W, LL Comb Ru	5.033	7.392		
+1.090D+0.750L+0.750S+0.5250E, LL C	6.499	7.388		
+1.090D+0.750L+0.750S+0.5250E, LL C	7.239	7.053		
+1.090D+0.750L+0.750S+0.5250E, LL C	7.101	8.473		
+0.60D+0.60W	2.038	2.509		
+0.470D+0.70E	3.947	2.906		
D Only	3.397	4.182		
L Only, LL Comb Run (*L)	-0.183	1.893		
L Only, LL Comb Run (L*)	0.804	1.446		
L Only, LL Comb Run (LL)	0.620	3.340		
S Only	1.561	0.939		
E Only	3.357	1.343		
H Only				

File: beam calcs with overstrength.ec6

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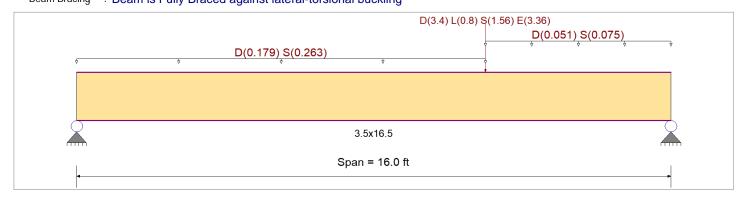
DESCRIPTION: BEAM B15

CODE REFERENCES

Calculations per NDS 2018, IBC 2018, CBC 2019, ASCE 7-16

Material Properties

Analysis Method : Allowable Stress Design	Fb+	2,400.0 psi	E : Modulus of Elasti	icity
	Fb -	1,850.0 psi	Ebend- xx	1,800.0ksi
	Fc - Prll	1,650.0 psi	Eminbend - xx	950.0ksi
Wood Species : DF/DF	Fc - Perp	650.0 psi	Ebend- yy	1,600.0ksi
Wood Grade : 24F-V4	Fv .	265.0 psi	Eminbend - yy	850.0ksi
	Ft	1,100.0 psi	Density	31.210 pcf
Beam Bracing : Beam is Fully Braced against lateral-torsional	al buckling	·	,	•



Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Beam self weight calculated and added to loads

Point Load: D = 3.40, L = 0.80, S = 1.560, E = 3.360 k @ 11.0 ft

 $\begin{array}{l} \mbox{Uniform Load}: \ D = 0.1790, \ S = 0.2630 \ k/ft, \ \mbox{Extent} = 0.0 \ -->> 11.0 \ ft, \ \mbox{Tributary Width} = 1.0 \ ft \\ \mbox{Uniform Load}: \ D = 0.0510, \ S = 0.0750 \ k/ft, \ \mbox{Extent} = 11.0 \ -->> 16.0 \ ft, \ \mbox{Tributary Width} = 1.0 \ ft \\ \mbox$

DESIGN SUMMARY					Design OK
Maximum Bending Stress Ratio Section used for this span	=	0.735 : 1 Ma 3.5x16.5	ximum Shear Stress Ratio Section used for this span	=	0.471 : 1 3.5x16.5
fb: Actual	=	2,027.87psi	fv: Actual	=	143.56 psi
Fb: Allowable	=	2,760.00psi	Fv: Allowable	=	304.75 psi
Load Combination Location of maximum on span Span # where maximum occurs	= =	+D+S 10.861ft Span # 1	Load Combination Location of maximum on span Span # where maximum occurs	= =	+D+S 14.657 ft Span # 1
Maximum Deflection Max Downward Transient Deflection Max Upward Transient Deflection Max Downward Total Deflection Max Upward Total Deflection	n	0.220 in Ratio = 0.000 in Ratio = 0.590 in Ratio = 0.000 in Ratio =	874 >= 360 0 < 360 325 >= 300 0 < 300		

Overall Maximum Deflections

Load Combination	Span	Max. "-" Defl	Location in Spar	Load Combination	Max. "+" Defl	Location in Span
+1.090D+0.750L+0.750S+0.5250E	1	0.5897	8.467		0.0000	0.000
Vertical Reactions			Sup	port notation : Far left is #1	Values in KIPS	
Load Combination		Suppo	rt 1 Support 2			
Overall MAXimum		5	291 7.042			
Overall MINimum		1.	050 2.310			
D Only		2.	495 3.330			
+D+L		2.	745 3.880			
+D+Lr		2.	495 3.330			
+D+S		4.	939 5.713			
+D+0.750Lr+0.750L		2.	582 3.742			
+D+0.750L+0.750S		4.	516 5.530			

6515 SE 30th St JayMarc Homes 154-21007 RJD 04-09-21

Wood Beam

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MULHERN & KULP STRUCTURAL ENGINEERING INC

Lic. # : KW-06004787 DESCRIPTION: BEAM B15

Vertical Reactions	Support notation : Far left is #1
vertical Reactions	Support notation . Lat lett is # i

Vertical Reactions		Suppo	ort notation : Far left is #1	Values in KIPS	
Load Combination	Support 1	Support 2			
+D+0.60W	2.495	3.330			
+1.126D+0.70E	3.544	5.366			
+D+0.750Lr+0.750L+0.450W	2.682	3.742			
+D+0.750L+0.750S+0.450W	4.516	5.530			
+1.090D+0.750L+0.750S+0.5250E	5.291	7.042			
+0.60D+0.60W	1.497	1.998			
+0.470D+0.70E	1.907	3.182			
D Only	2.495	3.330			
L Only	0.250	0.550			
S Only	2.445	2.383			
E Only	1.050	2.310			
H Only					

JAYMARC HOMES 6515 SE 30TH ST

MERCER ISLAND, WA

SHEAR WALL CALCULATIONS - WIND

REVIEWED BY: NJM

MARCH 26, 2021

PARAMETERS:

SINGLE FAMILY HOME

DESIGN WIND SPEED: 100 MPH

WIND EXPOSURE CATEGORY: C

SEISMIC DESIGN CATEGORY: D

CODE & DESIGN STANDARD: 2018 IBC CH. 1609, ASCE 7-16 CH. 26-30





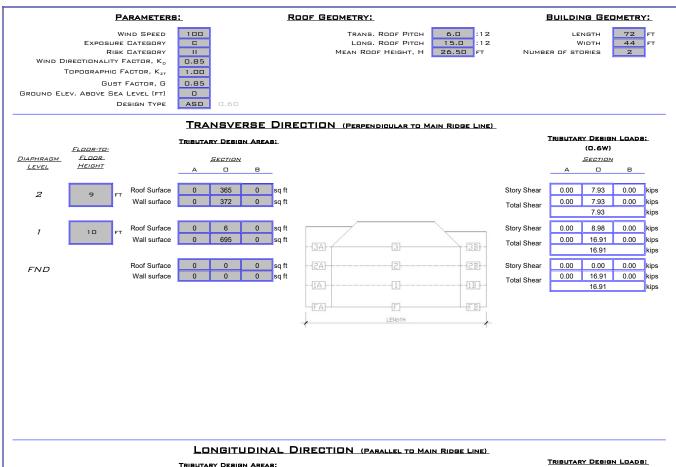
PROJECT NAME: 6515 SE 30TH ST

M&K PROJECT #: 154-21007

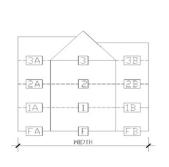
ENGINEER: RJD

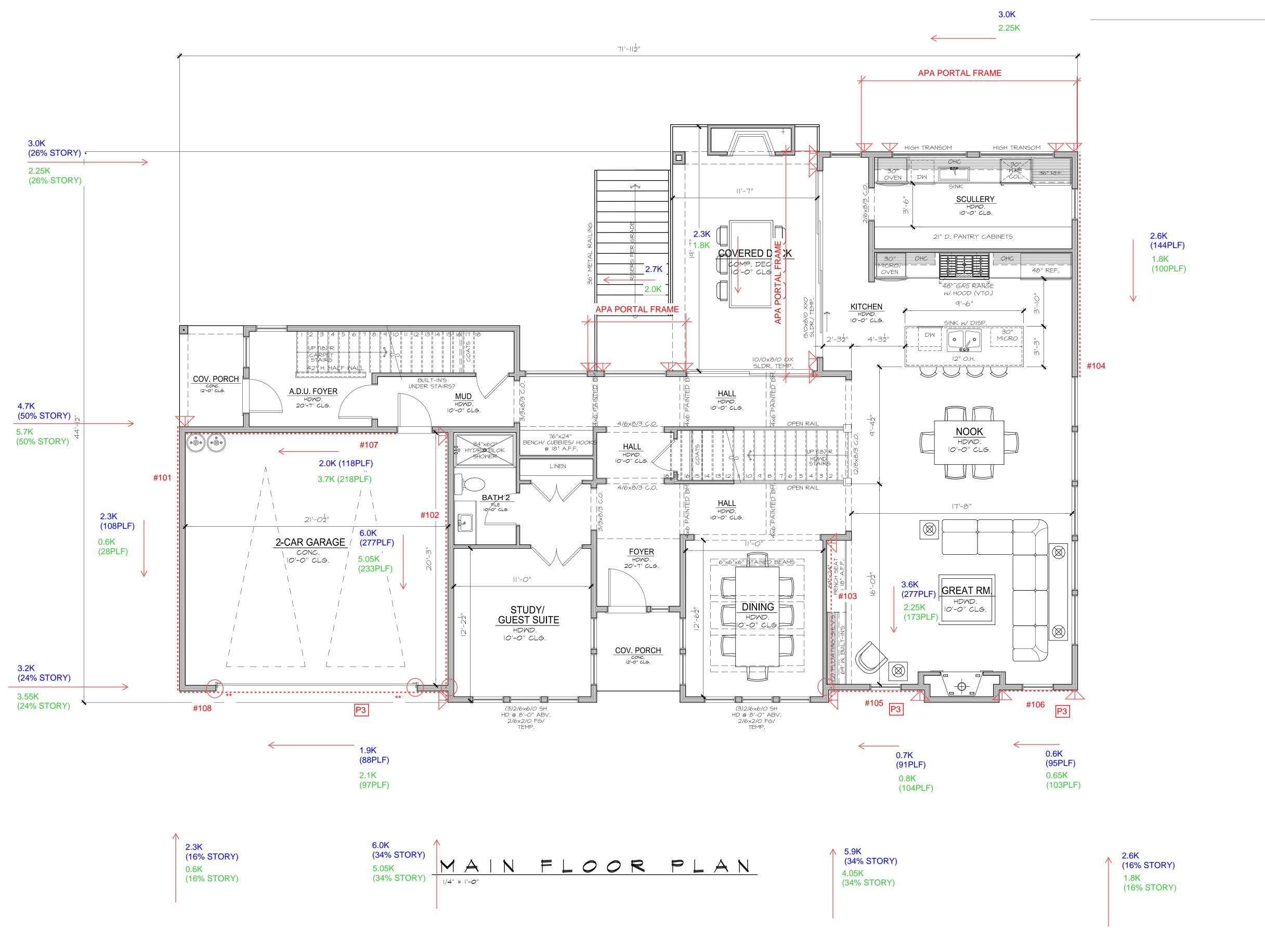
DATE: 26-MAR-21

WIND DESIGN SUMMARY PER ASCE 7-16



TRIBUTARY DESIGN AREAS: FLOOR-TO-DIAPHRAGM SECTION <u>FLOOR</u> HEIGHT LEVEL В 140 0 Roof Surface 0 sq ft 2 9 Wall surface 0 330 0 sq ft Roof Surface 0 0 0 saft 10 Wall surface 465 0 sq.ft 0 Roof Surface 0 0 0 FND Wall surface 0 0 0 sq ft





MAIN FLOOR PLAN NOTES

PLAN SPECIFIC 2015 MSEC. SECTION RO6

R406.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENTS (MANDATORY).

THIS RESIDENTIAL DWELLING SHALL COMPLY W/SUFFICIENT OPTIONS FROM

TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER OF CREDITS:

3.5 FOR a 1,501sf to 4,999sf HOME.

CREDITS PROVIDED IN THIS HOME AS FOLLOWS:

EFFICIENT BUILDING ENVELOPE IA: 0.5 CREDITS

PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 with FOLLOWING MODIFICATIONS:

VERTICAL FENESTRATION U = 0.28 WINDOWS FLOORS TO BE R-38 and SLAB ON GRADE TO BE R-10 PERIMETER and UNDER ENTIRE SLAB BELOW GRADE.

HIGH EFFICIENCY HVAC EQUIPMENT 3a: I.O CREDITS

GAS FURNACE WITH MINIMUM AFUE OF 94%

EFFICIENT WATER HEATING 5a: 0.5 CREDITS

ALL SHOWERHEAD and KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE SHALL BE RATED AT 1.75 GPM or LESS.

ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.

<u>EFFICIENT WATER HEATING 5c:</u>
1.5 CREDITS

WATER HEATING SYSTEM SHALL BE: GAS WATER HEATER WITH A MINIMUM EF OF O.91

WHOLE HOUSE VENTILATION

PROVIDE WHOLE HOUSE VENTILIATION per 2015 IRC. MI507 and IMC R403.8 USING LAUNRDY ROOM EXHAUST FAN INTEGRATED INTO FORCED ARE SYSTEM (FAU.) PROVIDE OUTDOOR FRESH AIR W/DUCTS CONNECTED TO THE RETURN SIDE OF THE AIR HANDLER.

SYMBOL LOCATION MIN. FAN REQUIREMENTS (ALL FANS VENT TO OUTDIDE)

BATH & POWDER Min. 50cfm. INTERMITTENT at .025mg per TABLE MI507.4

KITCHEN Min. 100cfm. INTERMITTENT at .025mg per TBL. MI507.4

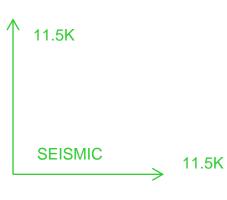
RANGE HOOD or DOWN DRAFT EXHAUST FAN RATED at min. 100cfm. at 0.10mg MAY BE USED FOR EXHAUST FAN REQMT. EXHAUST HOODS IN EXCESS OF 400cfm. SHALL BE INTERLOCKED AND PROVIDE MAKE UP AIR per w/MI503.4

C. LAUNDRY MIN. 360cfm. INTERMITTENT at .025mg TO FUNCTION 180cfm+ ROOM AS WHOLE HOUSE FAN (WHF.)

MECHANICAL CONTRACTOR TO SIZE WHF. FAN and SET OPERATING TIMER per TABLE MI507.3.3(1) FOR A 3,001-4,500sf. DWELLING w/4-5 BEDRMS. TO OPERATE INTERMITTENTLY and CONITINUOSLY per TABLE MI507.3.3(2)

PROVIDE CONTROLS FOR WHF. per MI507.3.2 AFFIX LABEL TO CONTROLS THAT READS "WHOLE HOUSE VENTILATION - SEE OPERATING INSTRUCTIONS"

↑ 16.9K WIND 10.9K



SQUARE FOOTAGE	E SUMMARY
MAIN FLOOR AREA + GARAGE	2,255 S.F.
UPPER FLOOR AREA	1,794 S.F.
TOTAL AREA	4,049 S.F.
COV'D PATIO	235 S.F.
COV'D PORCH	40 S.F.
TOTAL AREA UNDER ROOF	4,324 S.F.
OVERALL WIDTH	יל וו-'ול
OVERALL DEPTH	44' - 1 1/2

Updated : 03/09/2018

Method for Calculating Square Footage - ANSI Z765-2013 <u>except:</u> no separate distinction of labours-made or helphi-grade made and each level is measured to the



7525 SE 24th St., 487

Mercer Island, WA

425.266.9100

6515 SE 30th St.

Mercer Island, WA

plan name: -marketing name: -plan number: -mark sys. number:--

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC.) or those of the local municipality then the current standards and requirements of each respectively shall govern.

The drawings in this set are instruments of service and shall remain the property of JayMarc Homes, LLC.

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04.|5.2| | Submittal Date

Sheet Title/Description

JAYMARC HOMES

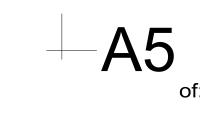
Design Firm

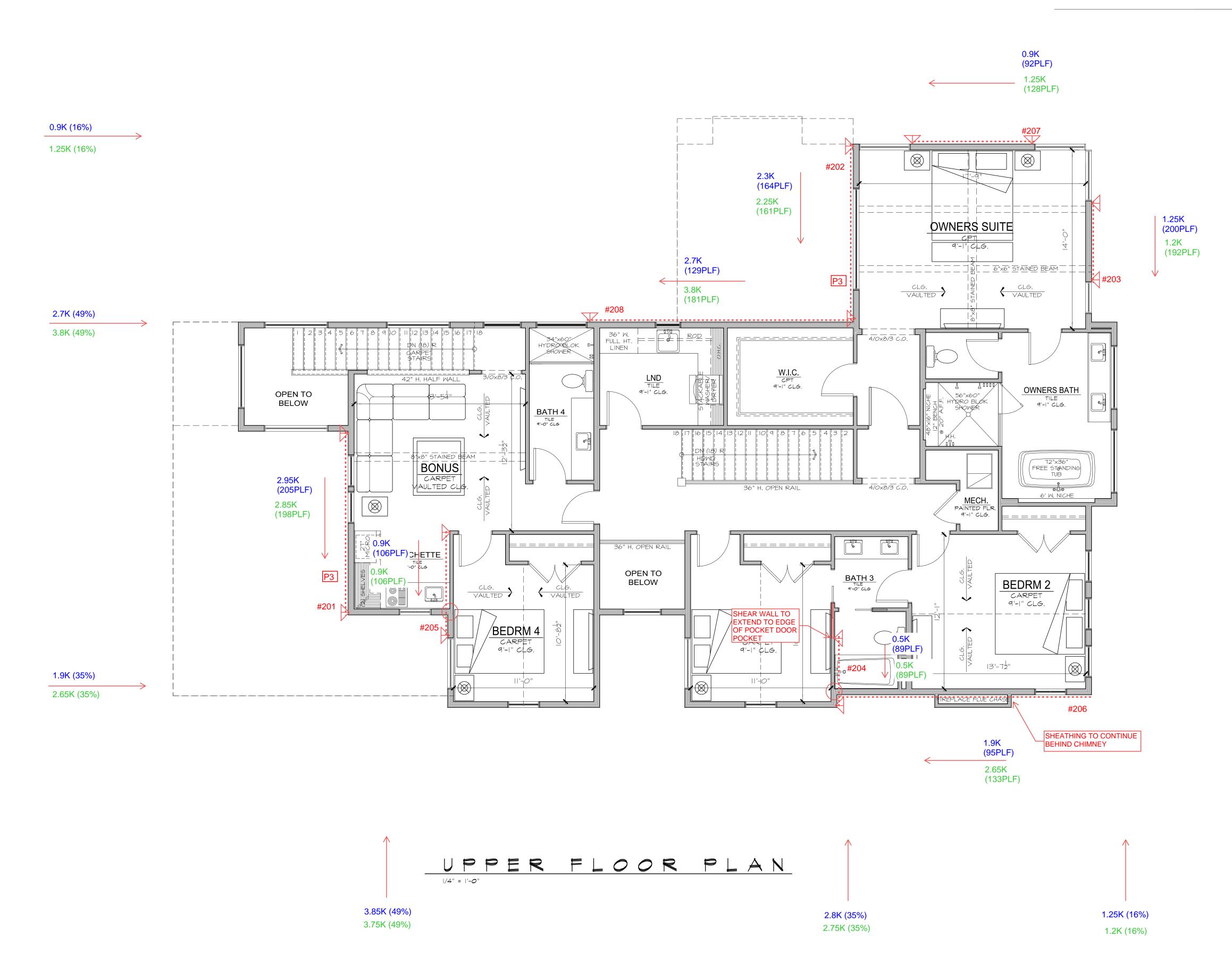
R.R. Drawn by:

R.R./ S.K. Checked by:

.

Primary Scale





UPPER FLOOR PLAN NOTES:

PLAN SPECIFIC 2015 WSEC. SECTION RO6

R406.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENTS (MANDATORY).

THIS RESIDENTIAL DWELLING SHALL COMPLY W/SUFFICIENT OPTIONS FROM TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER OF CREDITS:

3.5 FOR a 1,501sf to 4,999sf HOME. CREDITS PROVIDED IN THIS HOME AS FOLLOWS:

EFFICIENT BUILDING ENVELOPE Ia: 0.5 CREDITS

PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 with

FOLLOWING MODIFICATIONS:

VERTICAL FENESTRATION U = 0.28 WINDOWS

FLOORS TO BE R-38 and SLAB ON GRADE TO BE R-10 PERIMETER and UNDER ENTIRE SLAB BELOW GRADE.

HIGH EFFICIENCY HVAC EQUIPMENT 3a: I.O CREDITS

GAS FURNACE WITH MINIMUM AFUE OF 94%

EFFICIENT WATER HEATING 5a: 0.5 CREDITS

ALL SHOWERHEAD and KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE SHALL BE RATED AT 1.75 GPM or LESS.

ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.

EFFICIENT WATER HEATING 5c: 1.5 CREDITS

WATER HEATING SYSTEM SHALL BE:
GAS WATER HEATER WITH A MINIMUM EF OF 0.91

per w/M1503.4

MHOLE HOUSE VENTILATION

USING LAUNRDY ROOM EXHAUST FAN INTEGRATED INTO FORCED ARE SYSTEM (FAU.) PROVIDE OUTDOOR FRESH AIR W/DUCTS CONNECTED TO THE RETURN SIDE OF THE AIR HANDLER.

SYMBOL LOCATION MIN. FAN REQUIREMENTS (ALL FANS VENT TO OUTDIDE)

PROVIDE WHOLE HOUSE VENTILIATION per 2015 IRC. MI507 and IMC R403.8

BATH & Min. 50cfm. INTERMITTENT at .025wg per TABLE MI507.4

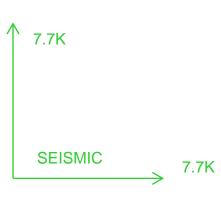
| KITCHEN | Min. 100cfm. INTERMITTENT at .025wg per TBL. MI507.4
| RANGE HOOD or DOWN DRAFT EXHAUST FAN RATED at min. 100cfm. at 0.10wg MAY BE USED FOR EXHAUST FAN REQMT. EXHAUST HOODS IN EXCESS OF 400cfm. SHALL BE INTERLOCKED AND PROVIDE MAKE UP AIR

C. LAUNDRY MIN. 180cfm. INTERMITTENT at .025mg TO FUNCTION
180cfm+ ROOM AS WHOLE HOUSE FAN (WHF.)
WHF.

MECHANICAL CONTRACTOR TO SIZE WHF. FAN and SET OPERATING TIMER PER
TABLE MI507.3.3(1) FOR A 3,001-4,500sf. DWELLING w/4-5 BEDRMS. TO OPERATE

INTERMITTENTLY and CONITINUOSLY per TABLE MI507.3.3(2)
PROVIDE CONTROLS FOR WHF. per MI507.3.2 AFFIX LABEL TO CONTROLS THAT READS "WHOLE HOUSE VENTILATION - SEE OPERATING INSTRUCTIONS"





P3 UNIT SHEAR: 630PLF/336PLF=1.88 89PLF*1.88 =167PLF 106PLF*1.88=199PLF

P3 UNIT SHEAR: 451PLF/239PLF=1.88 89PLF*1.88=167PLF 106PLF*1.88=199PLF

 SQUARE FOOTAGE SUMMARY

 MAIN FLOOR AREA + GARAGE
 2,255 S.F.

 UPPER FLOOR AREA
 1,794 S.F.

 TOTAL AREA
 4,049 S.F.

 COV'D PATIO
 235 S.F.

 COV'D PORCH
 40 S.F.

 TOTAL AREA UNDER ROOF
 4,324 S.F.

 OVERALL WIDTH
 71'-11 ½"

 OVERALL DEPTH
 44'-1 1/2"

 Updated: 03/09/2018

Method for Calculating Square Footage - ANSI Z765-2013 except: no separate distinction of 'above-grade or helphi-grade' areas and each level is measured to the

JAYMARC H O M E S

> 7525 SE 24th St., 487 Mercer Island, WA 98040 425.266.9100

. .

6515 SE 30th St. lercer Island, WA

plan name: –
marketing name: -plan number: -mark sys. number:--

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC.) or those of the local municipality then the current standards and requirements of each respectively shall govern.

The drawings in this set are instruments of service and shall remain the property of JayMarc Homes, LLC.

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04.l5.2l Submittal Date

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Sheet Title/Description

. JAYMARC HOMES

Design Firm R.R.

Drawn by:

R.R./ S.K.

Checked by:

Primary Scale





6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 201: 2ND - SIDE EXTERIOR BONUS
SHEARWALL PROPERTIES:
WALL HEIGHT, H 11.5 FT. MAX WALL OPENING HT, H _c 3.5 FT. WALL LENGTH, L 14.4 FT. QUALIFYING WALL LENGTH, L 9.3 FT. SHEARWALL ASSEMBLY P3
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 2950 LBS 5828 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P3 - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION: RESISTIVE DL 135 PLF OVERTURNING MOMENT 33.9 K-FT HOLD DOWN DESIGN LOAD 1533 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 11.9 K-FT HOLDOWN CAPACITY 1705 LBS
Hold-down Specification
SIMPSON CS16 STRAP TIE (14" END LENGTH)
SHEARWALL 202: 2ND - SIDE EXTERIOR OWNERS SUITE @ DECK SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 1.0 FT.
WALL LENGTH, L 14.0 FT. QUALIFYING WALL LENGTH, L 6.0 FT. SHEARWALL ASSEMBLY P3
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 2300 LBS 3780 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P3 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION: RESISTIVE DL 270 PLF OVERTURNING MOMENT 20.7 K-FT HOLD DOWN DESIGN LOAD 303 LBS DL AT ENDS OF WALL 70 LBS RESISTIVE MOMENT 16.5 K-FT HOLDOWN CAPACITY 1705 LBS
Hold-down Specification
SIMPSON CS16 STRAP TIE (14" END LENGTH)



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD
DATE: 26-MAR-21

SHEARWALL 203: 2ND - SIDE EXTERIOR OWNERS SUITE
SHEARWALL PROPERTIES:
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, H ₀ 0.0 FT. WALL LENGTH, L 6.3 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL 1250 LBS ALLOWABLE SHEARWALL CAPACITY 2100 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
Overturning Evaluation:
RESISTIVE DL 270 PLF OVERTURNING MOMENT 11.3 K-FT UPLIFT CONNECTOR DESIGN LOAD 1165 LBS DL AT ENDS OF WALL 215 LBS RESISTIVE MOMENT 4.0 K-FT HOLDOWN CAPACITY 1705 LBS
Hold-down Specification
SIMPSON CS16 STRAP TIE (14" END LENGTH)
SHEARWALL 204: 2ND - SIDE INTERIOR/EXTERIOR BATH 3
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT.
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT.
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 500 LBS < 1882 LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 GAPAGITY EVALUATION: TOTAL SHEAR LOAD ON WALL 500 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 0SB FASTENED W/ 8D NAILS AT 6 0.C. PANEL EDGES & 12 0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L S.6 FT. MAX WALL DPENING HT, HC MAX WALL DPENIN
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 500 LBS < 1882 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"0.C. PANEL EDGES & 12"0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 202 PLF OVERTURNING MOMENT 4.5 K-FT HOLD DOWN DESIGN LOAD 224 LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 500 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 OSB FASTENED W/ 8D NAILS AT 6 O.C. PANEL EDGES & 12 O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 202 PLF OVERTURNING MOMENT 4.5 K-FT HOLD DOWN DESIGN LOAD 224 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 3.2 K-FT HOLD DOWN DAPACITY 1705 LBS



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL	205: 2nd - Side Interior/Exterior Bed 4
SHEARWALL PROPER	ITIES:
WALL HEIGHT, H WALL LENGTH, L	9.0 FT. MAX WALL OPENING HT, H ₀ 0.0 FT. 8.5 FT. QUALIFYING WALL LENGTH, L 8.5 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION	on:
	TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 900 LBS 2856 LBS
,	SHEARWALL ASSEMBLY SPECIFICATION
	P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVAL	IATION!
RESISTIVE DL DL AT ENDS OF WALL	202 PLF OVERTURNING MOMENT 8.1 K-FT UPLIFT CONNECTOR DESIGN LOAD 396 LBS 70 LBS RESISTIVE MOMENT 4.7 K-FT HOLDOWN CAPACITY 1705 LBS
	Hold-down Specification
	SIMPSON CS16 STRAP TIE (14" END LENGTH)
l	
SHEARWALL	XXX: - NOT USED
SHEARWALL SHEARWALL PROPER	
SHEARWALL PROPER	O.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. SHEARWALL ASSEMBLY PO
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L	O.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. SHEARWALL ASSEMBLY PO
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L	D.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. SHEARWALL ASSEMBLY PO DN: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L	TOTAL SHEAR LOAD ON WALL D.D LBS #### #DIV/D! LBS
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATION	TOTAL SHEAR LOAD ON WALL BY HEAR LOAD ON WALL CHARLES SHEARWALL ASSEMBLY CHARLES SHEARWALL ASSEMBLY CHARLES SHEARWALL ASSEMBLY CHARLES SHEARWALL CAPACITY #### #DIV/O! LBS SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O!
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATION OVERTURNING EVALUATION	TIES: O.O. FT. MAX WALL OPENING HT, HC O.O. FT. QUALIFYING WALL LENGTH, L O.O. FT. QUALIFYING WALL LENGTH, L O.O. FT. SHEARWALL ASSEMBLY PO IN: TOTAL SHEAR LOAD ON WALL LBS #### #DIV/O! LBS SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O!
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATION	TOTAL SHEAR LOAD ON WALL BY HEAR LOAD ON WALL CHARLES SHEARWALL ASSEMBLY CHARLES SHEARWALL ASSEMBLY CHARLES SHEARWALL ASSEMBLY CHARLES SHEARWALL CAPACITY #### #DIV/O! LBS SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O!
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATION OVERTURNING EVALUATION RESISTIVE DL	TITES: O.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. SHEARWALL ASSEMBLY PO ON: TOTAL SHEAR LOAD ON WALL OBS #### #DIV/O! LBS SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O! JATION: O PLF OVERTURNING MOMENT #DIV/O! K-FT HOLD DOWN DESIGN LOAD OLBS RESISTIVE MOMENT O.O LBS RESISTIVE MOMENT O.O LBS
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATION OVERTURNING EVALUATION RESISTIVE DL	TIES: O.D FT. MAX WALL OPENING HT, HC O.D FT. O.D FT. QUALIFYING WALL LENGTH, L O.D FT. SHEARWALL ASSEMBLY PO ON: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY #### #DIV/O! LBS SHEARWALL ASSEMBLY SPECIFICATION PO - 1 - SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O! JATION: D PLF OVERTURNING MOMENT #DIV/O! K-FT HOLD DOWN DESIGN LOAD OLSS



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 206: 2ND - FRONT EXTERIOR BED2/BATH3
SHEARWALL PROPERTIES:
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, H ₀ 6.0 FT. WALL LENGTH, L 20.0 FT. QUALIFYING WALL LENGTH, L 17.5 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 1900 LBS 5796 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-side 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
Overturning Evaluation:
RESISTIVE DL 350 PLF OVERTURNING MOMENT 17.1 K-FT UPLIFT CONNECTOR DESIGN LOAD DESIGN LOAD LBS RESISTIVE MOMENT 44.4 K-FT HOLDOWN CAPACITY DESIGN LOAD LBS
HOLD-DOWN SPECIFICATION
No Holdown Required
SHEARWALL 207: 2ND - REAR EXTERIOR OWNERS SUITE
SHEARWALL PROPERTIES:
WALL HEIGHT, H 12.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 9.8 FT. QUALIFYING WALL LENGTH, L 9.8 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL 900 LBS ALLOWABLE SHEARWALL CAPACITY 3276 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION:
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 135 PLF OVERTURNING MOMENT 10.8 K-FT HOLD DOWN DESIGN LOAD 671 LBS
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 135 PLF OVERTURNING MOMENT 10.8 K-FT HOLD DOWN DESIGN LOAD 671 LBS DL AT ENDS OF WALL 70 LBS RESISTIVE MOMENT 4.3 K-FT HOLDOWN CAPACITY 1705 LBS



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 208: 2ND - REAR EXTERIOR LND/WIG	
SHEARWALL PROPERTIES:	
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, H $_{\rm c}$ 3.5 FT. WALL LENGTH, L 21.0 FT. QUALIFYING WALL LENGTH, L 19.0 FT. SHEAR	WALL ASSEMBLY P1
CAPACITY EVALUATION:	
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CA	APACITY
SHEARWALL ASSEMBLY SPECIFICATION	
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - ADEQUATE	EDGES BLOCKED
OVERTURNING EVALUATION: RESISTIVE DL 245 PLF OVERTURNING MOMENT 24.3 K-FT UPLIFT CONNECTE	
DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 37.5 K-FT HOI	LDOWN CAPACITY 1705 LBS
HOLD-DOWN SPECIFICATION	
No Holdown Required	
SHEARWALL XXX: - NOT USED	
SHEARWALL XXX: - NOT USED SHEARWALL PROPERTIES:	
SHEARWALL PROPERTIES: WALL HEIGHT, H	WALL ASSEMBLY PO
SHEARWALL PROPERTIES: WALL HEIGHT, H	WALL ASSEMBLY PD
SHEARWALL PROPERTIES: WALL HEIGHT, H	
SHEARWALL PROPERTIES: WALL HEIGHT, H	APACITY
SHEARWALL PROPERTIES: WALL HEIGHT, H	APACITY LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H	APACITY LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H	APACITY LBS - UNBLOCKED
SHEARWALL PROPERTIES: WALL HEIGHT, H	APACITY LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H	APACITY LBS - UNBLOCKED VN DESIGN LOAD LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H	APACITY LBS - UNBLOCKED VN DESIGN LOAD



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 101: 1ST - SIDE EXTERIOR GARAGE
SHEARWALL PROPERTIES:
WALL HEIGHT, H 11.0 FT. MAX WALL OPENING HT, H ₀ 0.0 FT. WALL LENGTH, L 21.2 FT. QUALIFYING WALL LENGTH, L 21.2 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 2300 LBS 7123 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION: RESISTIVE DL 135 PLF OVERTURNING MOMENT 25.3 K-FT UPLIFT CONNECTOR DESIGN LOAD 0 LBS DL AT ENDS OF WALL 800 LBS RESISTIVE MOMENT 28.4 K-FT HOLDOWN CAPACITY 0 LBS
HOLD-DOWN SPECIFICATION
No Holdown Required
SHEARWALL 102: 1st - Side Interior Garage
SHEARWALL 102: 1ST - SIDE INTERIOR GARAGE SHEARWALL PROPERTIES:
SHEARWALL PROPERTIES: WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 0.0 FT.
SHEARWALL PROPERTIES: WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1
SHEARWALL PROPERTIES: WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L 21.7 FT. MAX WALL OPENING HT, HC 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 6000 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L 21.7 FT. MAX WALL OPENING HT, HC 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 6000 LBS 7281 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 0SB
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L 21.7 FT. MAX WALL OPENING HT, HC 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 6000 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED
SHEARWALL PROPERTIES: WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 6000 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"0.C. PANEL EDGES & 12"0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 315 PLF OVERTURNING MOMENT 68.6 K-FT HOLD DOWN DESIGN LOAD 877 LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L WALL LENGTH, L TOTAL SHEAR LOAD ON WALL GOOD LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION:
SHEARWALL PROPERTIES: WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 6000 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"0.C. PANEL EDGES & 12"0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 315 PLF OVERTURNING MOMENT 68.6 K-FT HOLD DOWN DESIGN LOAD 877 LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 21.7 FT. SHEARWALL ASSEMBLY P1 GAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 6000 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 SB FASTENED W/ 8D NAILS AT 6 0.C. PANEL EDGES & 12 0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 315 PLF OVERTURNING MOMENT 68.6 K-FT HOLD DOWN DESIGN LOAD 877 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 49.6 K-FT HOLD DOWN CAPACITY 4935 LBS



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 103: 1st - Side Interior Great RM
SHEARWALL PROPERTIES:
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, H ₀ 0.0 FT. WALL LENGTH, L 13.0 FT. QUALIFYING WALL LENGTH, L 13.0 FT. SHEARWALL ASSEMBLY P1
GAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY
3600 LBS < 4368 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION: RESISTIVE DL 535 PLF OVERTURNING MOMENT 38.9 K-FT UPLIFT CONNECTOR DESIGN LOAD 427 LBS DL AT ENDS OF WALL 800 LBS RESISTIVE MOMENT 33.4 K-FT HOLDOWN CAPACITY 1705 LBS
HOLD-DOWN SPECIFICATION
SIMPSON CS16 STRAP TIE (14" END LENGTH)
SHEARWALL 104: 1ST - SIDE EXTERIOR KITCHEN/SCULLERY
SHEARWALL PROPERTIES:
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 7.0 FT. WALL LENGTH, L 18.0 FT. QUALIFYING WALL LENGTH, L 15.5 FT. SHEARWALL ASSEMBLY P1
GAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 2600 LBS 5166 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION: RESISTIVE DL 505 PLF OVERTURNING MOMENT 26.0 K-FT HOLD DOWN DESIGN LOAD 0 LBS DL AT ENDS OF WALL 470 LBS RESISTIVE MOMENT 54.2 K-FT HOLDOWN CAPACITY 0 LBS
Hold-down Specification
No Holdown Required



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL	xxx:	- NOT USED
SHEARWALL PROPE	RTIES:	
WALL HEIGHT, H WALL LENGTH, L		MAX WALL OPENING HT, H _c
CAPACITY EVALUATI	ON:	
	Tc	DTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY O LBS #### #DIV/O! LBS
		SHEARWALL ASSEMBLY SPECIFICATION
	FASTENED	PO - 1-SIDE 7/16" OSB D W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O!
Overturning Eval		
RESISTIVE DL DL AT ENDS OF WALL	DATION. D PLF D LBS	OVERTURNING MOMENT #DIV/O! K-FT UPLIFT CONNECTOR DESIGN LOAD OLBS RESISTIVE MOMENT O.O K-FT HOLDOWN CAPACITY OLBS
		HOLD-DOWN SPECIFICATION
		No Holdown Required
SHEARWALL 105: 1ST - FRONT EXTERIOR GREAT RM @ DINING		
		1ST - FRONT EXTERIOR GREAT RM @ DINING
SHEARWALL SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L	RTIES:	1ST - FRONT EXTERIOR GREAT RM @ DINING MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3
SHEARWALL PROPE	10.0 FT. 7.7 FT.	MAX WALL OPENING HT, HC 7.0 FT.
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L	7.7 FT.	MAX WALL OPENING HT, HC 7.0 FT.
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L	7.7 FT.	MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3 DTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L	TC	MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3 OTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 700 LBS 2713 LBS
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI	TIES: 10.0 FT. 7.7 FT. ON: FASTENED	MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3 OTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 700 LBS < 2713 LBS SHEARWALL ASSEMBLY SPECIFICATION P3 - 1-SIDE 7/16" DSB W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L	TIES: 10.0 FT. 7.7 FT. ON: FASTENED	MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3 OTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 700 LBS < 2713 LBS SHEARWALL ASSEMBLY SPECIFICATION P3 - 1-SIDE 7/16" DSB W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI OVERTURNING EVAL RESISTIVE DL	TIES: 10.0 FT. 7.7 FT. ON: FASTENED UATION: 480 PLF	MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3 TAL SHEAR LOAD ON WALL 700 LBS < 2713 LBS SHEARWALL ASSEMBLY SPECIFICATION P3 - 1-SIDE 7/16 OSB W/ 8D NAILS AT 3 O.C. PANEL EDGES & 12 O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING MOMENT 7.0 K-FT HOLD DOWN DESIGN LOAD LBS RESISTIVE MOMENT 12.2 K-FT HOLDOWN CAPACITY 4935 LBS
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI OVERTURNING EVAL RESISTIVE DL	TIES: 10.0 FT. 7.7 FT. ON: FASTENED UATION: 480 PLF	MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3 DTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 700 LBS < 2713 LBS SHEARWALL ASSEMBLY SPECIFICATION P3 - 1-SIDE 7/16" DSB W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE DVERTURNING MOMENT 7.0 K-FT HOLD DOWN DESIGN LOAD 0 LBS RESISTIVE MOMENT 12.2 K-FT HOLDOWN CAPACITY 4935 LBS HOLD-DOWN SPECIFICATION
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI OVERTURNING EVAL RESISTIVE DL	TIES: 10.0 FT. 7.7 FT. ON: FASTENED UATION: 480 PLF	MAX WALL OPENING HT, HC 7.0 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3 TAL SHEAR LOAD ON WALL 700 LBS < 2713 LBS SHEARWALL ASSEMBLY SPECIFICATION P3 - 1-SIDE 7/16 OSB W/ 8D NAILS AT 3 O.C. PANEL EDGES & 12 O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING MOMENT 7.0 K-FT HOLD DOWN DESIGN LOAD LBS RESISTIVE MOMENT 12.2 K-FT HOLDOWN CAPACITY 4935 LBS



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD
DATE: 26-MAR-21

SHEARWALL IUG: 1ST - FRONT EXTERIO GREAT RM				
SHEARWALL PROPERTIES:				
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, H _C 7.0 FT.				
WALL LENGTH, L 6.3 FT. QUALIFYING WALL LENGTH, L 4.0 FT. SHEARWALL ASSEMBLY P3				
BARAGITY EVALUATIONS				
CAPACITY EVALUATION: Total Shear Load on Wall Allowable Shearwall Capacity				
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 600 LBS 2048 LBS				
SHEARWALL ASSEMBLY SPECIFICATION				
P3 - 1-side 7/16" OSB				
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED				
<u>ADEQUATE</u>				
OVERTURNING EVALUATION:				
RESISTIVE DL 480 PLF OVERTURNING MOMENT 6.0 K-FT UPLIFT CONNECTOR DESIGN LOAD 0 LE				
DL AT ENDS OF WALL 800 LBS RESISTIVE MOMENT 8.7 K-FT HOLDOWN CAPACITY 4935 LE				
Hold-down Specification				
No Holdown Required				
SHEARWALL 107: 1st - Rear Interior Garage				
SHEARWALL PROPERTIES:				
WALL HEIGHT, H 11.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 17.0 FT. QUALIFYING WALL LENGTH, L 17.0 FT. SHEARWALL ASSEMBLY P1				
WALL LENGTH, L 17.0 FT. WOALFTING WALL LENGTH, L 17.0 FT. SHEARWALL ASSEMBLT FT				
CAPACITY EVALUATION:				
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY				
2000 LBS < 5712 LBS				
SHEARWALL ASSEMBLY SPECIFICATION				
P1 - 1-SIDE 7/16" OSB				
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED				
<u>ADEQUATE</u>				
OVERTURNING EVALUATION:				
RESISTIVE DL 440 PLF OVERTURNING MOMENT 22.0 K-FT HOLD DOWN DESIGN LOAD 0 LE				
DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 42.2 K-FT HOLDOWN CAPACITY 4935 LE				
Hai D-Dawn Specification				
Hold-down Specification				
No Holdown Required				
140 Hoebawa Kedalikeb				
No Holds W Regards				



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL	108: 1st - Front Exterior Garage	
SHEARWALL PROPERTIES:		
WALL HEIGHT, H WALL LENGTH, L	10.0 FT. MAX WALL OPENING HT, H _c 8.0 FT. 21.6 FT. QUALIFYING WALL LENGTH, L 5.5 FT.	SHEARWALL ASSEMBLY P3
CAPACITY EVALUATI	<u>ı:</u>	
	TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARW 1900 LBS 2945	
7	SHEARWALL ASSEMBLY SPECIFICATION	-
	P3 - 1-SIDE 7/16" OSB fastened w/ 8d nails at 3"d.c. panel edges & 12"d.c. panel f adequate	TIELD - EDGES BLOCKED
OVERTURNING EVAL	TION	
RESISTIVE DL DL AT ENDS OF WALL		NNECTOR DESIGN LOAD ULBS HOLDOWN CAPACITY ULBS
	Hold-down Specification	
	No Holdown Required	
SHEARWALL	XXX: - Not Used	
SHEARWALL SHEARWALL PROPER		
		SHEARWALL ASSEMBLY PO
SHEARWALL PROPE	O.D FT. MAX WALL OPENING HT, HC O.D FT. O.D FT. QUALIFYING WALL LENGTH, L O.D FT.	SHEARWALL ASSEMBLY PD
SHEARWALL PROPE WALL HEIGHT, H WALL LENGTH, L	O.D FT. MAX WALL OPENING HT, HC O.D FT. O.D FT. QUALIFYING WALL LENGTH, L O.D FT.	ALL CAPACITY
SHEARWALL PROPE WALL HEIGHT, H WALL LENGTH, L	IES: O.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. I: Total Shear Load on Wall Allowable Shearw	VALL CAPACITY
SHEARWALL PROPE WALL HEIGHT, H WALL LENGTH, L	IES: O.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. I: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARW OLES #### #DIV/E	VALL CAPACITY D! LBS
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI	TOTAL SHEAR LOAD ON WALL SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL #DIV/O!	VALL CAPACITY D! LBS
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI OVERTURNING EVAL	TOTAL SHEAR LOAD ON WALL SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL #DIV/O!	VALL CAPACITY D! LBS - FIELD - UNBLOCKED
SHEARWALL PROPEI WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI	ES: O.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. II: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARW O LBS #### #DIV/C SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL #DIV/O!	VALL CAPACITY D! LBS
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI OVERTURNING EVAL RESISTIVE DL	ES: O.D FT. MAX WALL OPENING HT, HC O.D FT. O.D FT. QUALIFYING WALL LENGTH, L O.D FT. II TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARW D LBS #### #DIV/C SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL #DIV/O! ATION: O PLF OVERTURNING MOMENT #DIV/O! K-FT HOI	VALL CAPACITY D! LBS FIELD - UNBLOCKED LD DOWN DESIGN LOAD
SHEARWALL PROPER WALL HEIGHT, H WALL LENGTH, L CAPACITY EVALUATI OVERTURNING EVAL RESISTIVE DL	ES: O.O FT. MAX WALL OPENING HT, HC O.O FT. O.O FT. QUALIFYING WALL LENGTH, L O.O FT. IL TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARW O LBS #### #DIV/C SHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL #DIV/O! STION: O PLF OVERTURNING MOMENT #DIV/O! K-FT HOLD O LBS RESISTIVE MOMENT O.O K-FT	VALL CAPACITY D! LBS FIELD - UNBLOCKED LD DOWN DESIGN LOAD

JAYMARC HOMES 6515 SE 30TH ST

MERCER ISLAND, WA

SHEAR WALL CALCULATIONS - SEISMIC

REVIEWED BY: NJM

MARCH 26, 2021

PARAMETERS:

SINGLE FAMILY HOME

DESIGN WIND SPEED: 100 MPH

WIND EXPOSURE CATEGORY: C

SEISMIC DESIGN CATEGORY: D

CODE & DESIGN STANDARD: 2018 IBC CH. 1609, ASCE 7-16 CH. 26-30





PROJECT NAME: 6515 SE 30TH ST

JAYMARC HOMES

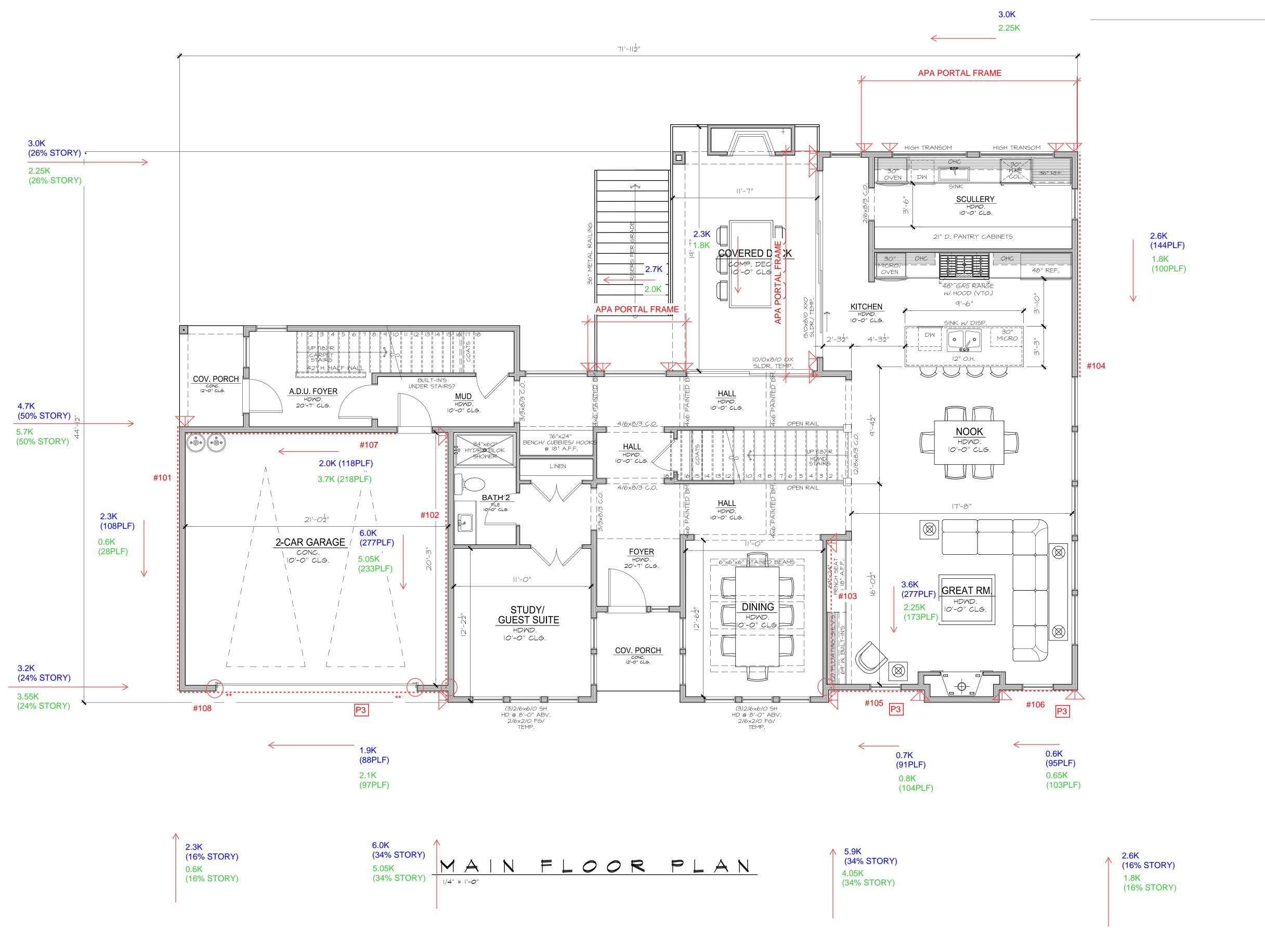
M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SEISMIC CALCULATION - ASCE 7-16

1 10.0 2000 15 15.4 45 49 49 49 49 49 49 4	Direct Industry Direct Ind	<u>USER INPUTS:</u> SITE CLASS SPECTRAL RESPON				
BITE DIAGE	Description	SITE CLASS SPECTRAL RESPON	iory:			IINATION:
SPECTMAL RESPONSE ACCELERATION 1.0 SEC. 81 1.4815 1.0815 1	SHECTHAL RESPONDER AGESLERATION 1.0 SEC. 81 DEBUTARY CATEGORY 10 10 10 10 10 10 10 1	SPECTRAL RESPON			USER INPUTS:	
BPECTRAL RESPONSE AGGLERATION 1.0 SEC. 8 0.099	DECOMMEND ACCELERATION 1, D SEC. \$1 1.400 1.100			D	BUILDING PERIOD COEFFICIENT,	C, 0.020
BPECTRAL RESPONSE AGGLERATION 1.0 SEC. 8 0.099	DECIDIAL RESPONSE ACCELERATION 1.0 BCC 81 DARK		SE ACCELERATION 0.2 SEC. SS	1.405	I DNG-PERIOD TRANS PERIOD "	T. (SEC) 6
	DECUMENT PARTIES APPRISON TABLE					_
APPRINCE STEE CORPTICIONT, PA 1.20 To 0.102	MARNING REPORTED ALLEGED 1.0 1	SPECTRAL RESPON	SE ACCELERATION 1.0 SEC, S1	0.489	HT. ABV BASE TO HIGHEST LEVE	EL, N _N 19
MARRIERE	APPROXIMATE FUNDAMENTAR, PRINCE, T. 1.120 To 1.121 To 1.122 To 1.122 To 1.123 To 1.124 To 1.12	OCCUPANCY CATEG	IORY	II	CALCULATED VALUES:	
Site Coefficient, FA 1.20 To 0.10%	BITE CONTINUENT, FA				·	IDD T D 182
STE DISPETIBILITY, FV	SITE CORPTIONER, For					
Design appendix Design According Design	Design Section	SITE COEFFICIENT,	FA	1.20	Т	0.105
MANNUM SPECTRAL RESPONSE ADDICERATION, 8, 1,080 DESIGN SPECTRAL RESPONSE ADDICERATION, 8, 1,124 DESIGN SPECTRAL RESPONSE ADDICERATION, 8, 1,090 THE SITE CLASS HAVE SESSION TO BE C	MAXMAM SPECTRAL RESPONSE ADDILERATION, 8	SITE COEFFICIENT,	FV	1.81	Ts	0.525
MAXIMUM SPECTIAL RESPONSE ADDICERATION, 8 0.086	MANIMUM SPECTIAL REPORTER ADDICEARTION, 8 ₁₀ DESIGN APECTRAL REPORTER ADDICEARTION, 8 ₁₁ DESIGN APECTRAL REPORTER ADDICEARTION, 8 ₁₂ DESIGN APECTRAL REPORTER ADDICEARTION, 8 ₁₄ DESIGN APECTRAL REPORTER ADDICEARTION, 8 ₁₄ DESIGN APECTRAL REPORTER ADDICEARTION, 8 ₁₄ DESIGN APECTRAL REPORTER ADDICEARTIN, 8 ₁₄ DESIGN APECTRAL REPORT ADDICEARTIN, 8 ₁₄	CALCULATED VALUE	<u>s:</u>			
DEBIN SPECTRAL REPONSE AGGLERATION, 8, D.980	Design protectal response acceleration, Bar Design protectal response acceleration, Design protectal				SPECTRAL RESPONSE ACC., S, (G)	1.124
DESIGN SPECTMAL RESPONSE ACCILERATION, Sign 1.124 0.500 0.50	DESIGN SPECTMAL RESPONSE ADDRESSATION, 8 1,124 0,590 1,124 0,590 THE SECRET OF THE SECTION 11,4; PRESENTED CONTROL OF THE S					
DESIGN BPECTRAL RESPONSE ACCELERATION Bot D. D. D. TO BE D	Design Brettman Legenburk Angelerration, 8g 0.500 Yes Pre-ARGE 7-16 Section 11-4.	MAXIMUM SPECTRA	_ RESPONSE ACCELERATION, B _{M1}	0.886		
SCIBMIC DESIGN CATEGORY (SHORT TERM) D	Science Design Dated by Island Team D	DESIGN SPECTRAL	RESPONSE ACCELERATION, S _{DS}	1.124	SITE CLASS ASSUMPTION	
Science Design Attroducy (1-0 section trens) D	Science Design Cartegory (1-0 per proper) Design Cartegory (DESIGN SPECTRAL	RESPONSE ACCELERATION, S _{D1}	0.590		
SEISHUE DESIRE DATEORY (1.0 SECOND TERM) DEAD LOAD CALCULATION: TOTAL LEVEL (MPS) TOTAL L	SEISHOU DESIGN CATEGORY (1.0 SCOND TERM) EQUIVALENT LATERAL TORDE PRODEDURE DEAD LODG CALCULATION: 1 10,0 20,0 15 15,4 45 2 9,0 2500 17 5.8 45 3 0,0 0 0 0 0 0 4 0,0 0 0 0 0 0 5 0,0 0 0 0 0 0 6 0,0 0 0 0 0 0 7 0,0 0 0 0 0 0 0 8 0,0 0 0 0 0 0 0 9 0,0 0 0 0 0 0 9 0,0 0 0 0 0 0 10 0,0 0 0 0 0 0 11 0,0 0 0 0 0 0 12 0,0 0 0 0 0 0 13 0,0 0 0 0 0 0 14 0,0 0 0 0 0 0 15 0,0 0 0 0 0 16 0,0 0 0 0 0 17 0,0 0 0 0 0 18 0,0 0 0 0 0 19 0,0 0 0 0 0 10 0,0 0 0 0 0 11 0,0 0 0 0 0 12 0,0 0 0 0 0 13 0,0 0 0 0 0 14 0,0 0 0 0 0 15 0,0 0 0 0 16 0,0 0 0 0 17 0,0 0 0 0 18 0,0 0 0 0 19 0,0 0 0 0 10 0,0 0 0 11 0,0 0 0 0 12 0,0 0 0 0 13 0,0 0 0 0 14 0,0 0 0 0 15 0,0 0 0 16 0,0 0 0 17 0,0 0 0 18 0,0 0 0 19 0,0 0 0 10 0,0 0 0 10 0,0 0 0 11 0 0,0 0 12 0,0 0 0 13 0,0 0 0 14 0,0 0 0 15 0,0 0 0 16 0,0 0 0 17 0,0 0 0 18 0,0 0 0 19 0,0 0 0 10 0,0 0 0 10 0,0 0 0 11 0 0 0 0 12 0,0 0 0 13 0,0 0 0 14 0,0 0 0 15 0,0 0 0 16 0,0 0 0 17 0,0 0 0 18 0,0 0 0 19 0,0 0 0 10 0,0 0 0 10 0,0 0 0 0 10 0,0 0 0 0 10 0,0 0 0 10 0,0 0 0 0 10 0,0 0 0 0 10 0,0 0 0 0 10 0,0 0 0 0 0 10 0,0 0 0 0 0 10 0,0 0 0 0 0 10 0,0 0 0 0 0 10 0,0 0 0 0 0 10 0,0 0 0 0 0	Scienic Decien C	ATEGORY (GHORT TERM)	D		SS MAY BE ASSUMED
Dead Load Dakument	Dead Load Calculation:				ID BE D	
DEAD FOR MALL TRIB. TOTAL LEVEL STORY HT. [FT.] ASEA [FT.] DEAD LOAD (MRT) TOLLYEL (SIME) TOTAL LEVEL 1 10.0 2000 15 5 6.6.8 45 45 45 45 45 45 45 4	Dead Load Calculation:					
		<u>EQUIVALENT LATER</u>	AL FORCE PROCEDURE			
LEVEL STORY HT. (FL.) ABCA (FT.) DEAD LIANS (FEET STORY STREAM PROPERTY C. C. C. C. C. C. C. C	TOTAL LEVEL STORY HIT STORY HIT PARA LODG (REP) TOTAL LEVEL STORY HIT STORY	DEAD LOAD CALCUL	ATION:			
1	1 10.0 2000 15 15.4 49 49 49 49 49 49 49				DL OF EXT WALL TRIB.	
1	1	LEVEL	STORY HT. (FT.) AREA (FT ²)	DEAD LOAD (PSF)		TOTAL LEVEL D
2 9.0 2500 17 6.8 40 0.0	2 9.0 2500 17 6.8 49 49 49 6 6 6 6 6 6 6 6 6				15.4	
3	S					
4 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	## 10.0 0 0 0 0 0 0 0 0 0	=				
\$ 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	S	-				
7 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7	5	0.0		0.0	
B	S	6	0.0		0.0	
9	9 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.	7	0.0		0.0	
10	10					
1	1					
12	12					
13	13					
14	14					
15	19					
17	17		0.0		0.0	
18	18	16	0.0		0.0	
19	19	17	0.0		0.0	
TRANSVERSE LONGITUDINAL STANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL STORY SHEAR DALDULATION: DISTRIBUTION EXPONENT. TOTAL DEAD LOAD OF STRUCTURE. 95 ALLOWABLE LOADS TRANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL STORY SHEAR F.	TOTAL DEAD LOAD OF STRUCTURE 95	18			0.0	
TOTAL DEAD LOAD OF STRUCTURE 95	Transverse					
SEISMIG RESPONSE COEFFICIENT: TRANSVERSE LONGITUDINAL	RESPONSE MODIFICATION FACTOR, R 6.5 6.5	20	0.0		□.□	
SEISMIC RESPONSE COEFFICIENT: TRANSVERSE LONGITUDINAL RESPONSE MODIFICATION FACTOR, R 6.5 6.5	RESPONSE MODIFICATION FACTOR, R 6.5 6.5				TOTAL DEAD LOAD DE STE	RUCTURE 95
RESPONSE MODIFICATION FACTOR, R	RESPONSE MODIFICATION FACTOR, R 6.5 6.5	SEISMIC RESPONSE	COEFFICIENT:		<u> </u>	35
Codepancy Importance Factor, I	DGCUPANCY IMPORTANCE FACTOR,	<u> </u>		LONGITUDINAL		
COUDINATE INFORMATION COURT CO	DCCUPANCY IMPORTANCE FACTOR,	RESPONSE MODIFICA	TION FACTOR. R 6.5	6.5		
BASE SHEARS: ULTIMATE LOADS	Seismic Response Color C					
BASE SHEARS: LITIMATE LOADS	Start Shears	OCCUPANCY IMPORTA	NCE FACTOR, IE 1.00	1.00		
TRANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00 Comparison Comp	TRANSVERSE LONGITUDINAL 16 K 16 K 11.5 K 11.5 K 11.5 STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00 VERT, DIST, TRANSVERSE LONGITUDINAL EVEL PACTURE TOWN 10 0.326 S.33 K 5.3 K 5.3 K 7.7	SEISMIC RESPONSE	COEFFICIENT, C. 0.173	0.173		
TRANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00 Comparison Comp	TRANSVERSE LONGITUDINAL 16 K 16 K 11.5 K 11.5 K 11.5 STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00 VERT, DIST, TRANSVERSE LONGITUDINAL EVEL PACTURE TOWN 10 0.326 S.33 K 5.3 K 5.3 K 7.7	BASE CHEARS	III TIMATE LOADS	V D 7 =	ALL OWARLE LOADS	
Transverse	16	DASE BHEARS.	·			
STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00 Vert. Dist. Transverse Longitudinal Transverse Story Shear, F. Story Sh	STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00 Vert. Dist. Transverse Longituonal Story Shear, F. Story		IRANSVERSE LONGITUDINAI	<u> </u>	IRANSVERSE	LONGITUDINAL
Note	Note		16 K 16	К	11.5 K	11.5
Note	Note	STORY SHEAR CALC	ULATION:			
Note	Nert Dist. Fander					
Vert. Dist. Transverse Longitudinal Transverse Story Shear, F. Story She	Transverse Conditional Transverse Condit	DISTRIBUTION EXPONENT	1.55			
EVEL FARTER Down STORY SHEAR, F., STORY	Tantor T					
1 0.326 5.3 k 5.3 k 3.7 k 11.5 k 7.7 k 0.0	1 0.326 5.3 K 5.3 K 3.7 K 11.5 K 3.7 K 11.5 K 3.7 K 11.5 K 3.7 K 7.7 K 0.0 <					
2 0.674 11.0 K 11.0 K 7.7 K 0.0 K 0.0 <t< td=""><td>2 0.674 11.0 K 11.0 K 7.7 A 7.7 K 7.7 K 7.7 K 7.7 K 0.0 <t< td=""><td></td><td></td><td></td><td></td><td></td></t<></td></t<>	2 0.674 11.0 K 11.0 K 7.7 A 7.7 K 7.7 K 7.7 K 7.7 K 0.0 K 0.0 <t< td=""><td></td><td></td><td></td><td></td><td></td></t<>					
3 0.000 0.0 K 0.0	3 0.000 0.0 K 0.0					
4 0.000 0.0 K 0.0	4 0.000 0.0 K <	1 0.326				
5 0.00 0.0 K	5 0.00 K 0.0	1 0.326 2 0.674				
6	6 0.00 0.0 K	1 0.326 2 0.674 3 0.000	0.0 K 0.0			
8 0.00 0.0 K 0.0	8 0.00 0.0 K 0.0	1 0.326 2 0.674 3 0.000 4 0.000		К 0.0	. 0.0	, OO
9 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 C C C C C C C C C C C C C C C C C C	9 0.00 0.0 K 0.0	1 0.326 2 0.674 3 0.000 4 0.000 5 0.00 6 0.00	о.о к о.о			~ 0.0
10 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 C C 0.0 C	10 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 12 0.0 C C C C C C C C C C C C C C C C C C	1 0.326 2 0.674 3 0.000 4 0.000 5 0.00 6 0.00 7 0.00	0.0 K 0.0 C C C C C C C C C C C C C C C C C C	K 0.0	о к о.о к о.о о к о.о к о.о	к 0.0
11 0.00 0.0 K 0.0	11 0.00 0.0 K 0.0	1 0.326 2 0.674 3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00	0.0 K 0.0 0.0 K 0.0 0.0 K 0.0	к 0.0 к 0.0	о к о о к о о о к о о о о к о о о о о о	к о.о к о.о
12	12 0.00 0.0 K 0.0	1 0.326 2 0.674 3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00	0.0 K 0.0 0.0 K 0.0 0.0 K 0.0 0.0 K 0.0	к 0.0 к 0.0 к 0.0	0 K 0.0 K 0.0 0 K 0.0 K 0.0 0 K 0.0 K 0.0	к о.о к о.о
13	13 0.00 0.0 K 0.0	1	0.0 K 0.0	к 0.0 к 0.0 к 0.0 к 0.0	D K D.D K D.C	к о.о к о.о к о.о к о.о
14 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0	14 0.00 0.0 K 0.0	1 0.326 2 0.674 3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00 10 0.00	0.0 K 0.0	к	CO K O.O K O.C C K O.O K O.C	1
	15 0.00 0.0 K 0.0 K 0.0 K 0.0 16 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 17 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 18 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0	1	0.0 K 0.0 C C C C C C C C C C C C C C C C C C	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0	0	K 0.0
	16 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 17 0.00 0.0 K 0.0	1	0.0 K 0.0	К О.С К О.С К О.С К О.С К О.С К О.С	CO K O.O K O.C C K O.O K O.C	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0
16 0.00 K 0.0 K 0.0 K 0.0 K 0.0	18 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0	1	0.0 K 0.0	K 0.0.0 K 0.0.	D K D.D K D.C	K 0.0
		1	0.0 K 0.0	K 0.C	CO K O.O K O.C C K O.O K O.C	K 0.0
	19 0.00 K 0.0 K 0.0 K 0.0	1	0.0 K 0.0 C C C C C C C C C C C C C C C C C C	K 0.C	CO K O.O K O.C	K 0.0
		1	0.0 K 0.0	K 0.C	CO K O.O K O.C C C C C C C C C C C C C C C C C C C C	K 0.0
	20 0.00 K 0.0 K 0.0 K 0.0 K 0.0	1	0.0 K 0.0 C C C C C C C C C C C C C C C C C C	K 0.C		K



MAIN FLOOR PLAN NOTES

PLAN SPECIFIC 2015 MSEC. SECTION RO6

R406.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENTS (MANDATORY).

THIS RESIDENTIAL DWELLING SHALL COMPLY W/SUFFICIENT OPTIONS FROM

TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER OF CREDITS:

3.5 FOR a 1,501sf to 4,999sf HOME.

CREDITS PROVIDED IN THIS HOME AS FOLLOWS:

EFFICIENT BUILDING ENVELOPE IA: 0.5 CREDITS

PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 with FOLLOWING MODIFICATIONS:

VERTICAL FENESTRATION U = 0.28 WINDOWS FLOORS TO BE R-38 and SLAB ON GRADE TO BE R-10 PERIMETER and UNDER ENTIRE SLAB BELOW GRADE.

HIGH EFFICIENCY HVAC EQUIPMENT 3a: I.O CREDITS

GAS FURNACE WITH MINIMUM AFUE OF 94%

EFFICIENT WATER HEATING 5a: 0.5 CREDITS

ALL SHOWERHEAD and KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE SHALL BE RATED AT 1.75 GPM or LESS.

ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.

<u>EFFICIENT WATER HEATING 5c:</u>
1.5 CREDITS

WATER HEATING SYSTEM SHALL BE: GAS WATER HEATER WITH A MINIMUM EF OF O.91

WHOLE HOUSE VENTILATION

PROVIDE WHOLE HOUSE VENTILIATION per 2015 IRC. MI507 and IMC R403.8 USING LAUNRDY ROOM EXHAUST FAN INTEGRATED INTO FORCED ARE SYSTEM (FAU.) PROVIDE OUTDOOR FRESH AIR W/DUCTS CONNECTED TO THE RETURN SIDE OF THE AIR HANDLER.

SYMBOL LOCATION MIN. FAN REQUIREMENTS (ALL FANS VENT TO OUTDIDE)

BATH & POWDER Min. 50cfm. INTERMITTENT at .025mg per TABLE MI507.4

KITCHEN Min. 100cfm. INTERMITTENT at .025mg per TBL. MI507.4

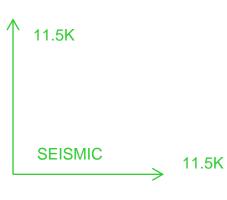
RANGE HOOD or DOWN DRAFT EXHAUST FAN RATED at min. 100cfm. at 0.10mg MAY BE USED FOR EXHAUST FAN REQMT. EXHAUST HOODS IN EXCESS OF 400cfm. SHALL BE INTERLOCKED AND PROVIDE MAKE UP AIR per w/MI503.4

C. LAUNDRY MIN. 360cfm. INTERMITTENT at .025mg TO FUNCTION 180cfm+ ROOM AS WHOLE HOUSE FAN (WHF.)

MECHANICAL CONTRACTOR TO SIZE WHF. FAN and SET OPERATING TIMER per TABLE MI507.3.3(1) FOR A 3,001-4,500sf. DWELLING w/4-5 BEDRMS. TO OPERATE INTERMITTENTLY and CONITINUOSLY per TABLE MI507.3.3(2)

PROVIDE CONTROLS FOR WHF. per MI507.3.2 AFFIX LABEL TO CONTROLS THAT READS "WHOLE HOUSE VENTILATION - SEE OPERATING INSTRUCTIONS"

↑ 16.9K WIND 10.9K



SQUARE FOOTAGE	E SUMMARY
MAIN FLOOR AREA + GARAGE	2,255 S.F.
UPPER FLOOR AREA	1,794 S.F.
TOTAL AREA	4,049 S.F.
COV'D PATIO	235 S.F.
COV'D PORCH	40 S.F.
TOTAL AREA UNDER ROOF	4,324 S.F.
OVERALL WIDTH	יל וו-'ול
OVERALL DEPTH	44' - 1 1/2

Updated : 03/09/2018

Method for Calculating Square Footage - ANSI Z765-2013 <u>except:</u> no separate distinction of labours-made or helphi-grade made and each level is measured to the



7525 SE 24th St., 487

Mercer Island, WA

425.266.9100

6515 SE 30th St.

Mercer Island, WA

plan name: -marketing name: -plan number: -mark sys. number:--

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC.) or those of the local municipality then the current standards and requirements of each respectively shall govern.

The drawings in this set are instruments of service and shall remain the property of JayMarc Homes, LLC.

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04.|5.2| | Submittal Date

Sheet Title/Description

JAYMARC HOMES

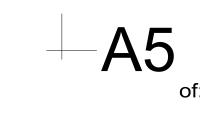
Design Firm

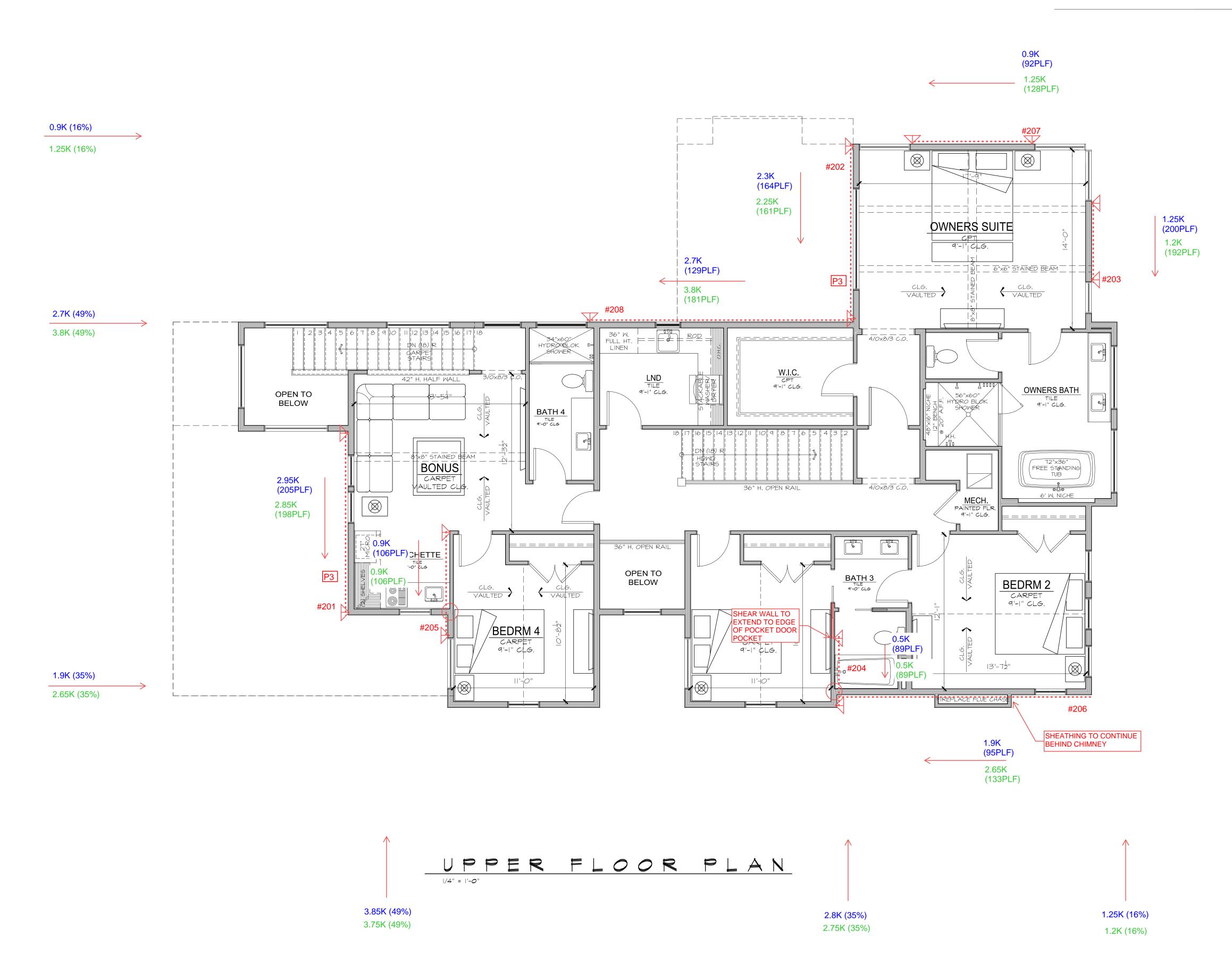
R.R. Drawn by:

R.R./ S.K. Checked by:

.

Primary Scale





UPPER FLOOR PLAN NOTES:

PLAN SPECIFIC 2015 WSEC. SECTION RO6

R406.2 ADDITIONAL ENERGY EFFICIENCY REQUIREMENTS (MANDATORY).

THIS RESIDENTIAL DWELLING SHALL COMPLY W/SUFFICIENT OPTIONS FROM TABLE R406.2 TO ACHIEVE THE FOLLOWING MIN. NUMBER OF CREDITS:

3.5 FOR a 1,501sf to 4,999sf HOME. CREDITS PROVIDED IN THIS HOME AS FOLLOWS:

EFFICIENT BUILDING ENVELOPE Ia: 0.5 CREDITS

PRESCRIPTIVE COMPLIANCE IS BASED ON TABLE R402.1.1 with

FOLLOWING MODIFICATIONS:

VERTICAL FENESTRATION U = 0.28 WINDOWS

FLOORS TO BE R-38 and SLAB ON GRADE TO BE R-10 PERIMETER and UNDER ENTIRE SLAB BELOW GRADE.

HIGH EFFICIENCY HVAC EQUIPMENT 3a: I.O CREDITS

GAS FURNACE WITH MINIMUM AFUE OF 94%

EFFICIENT WATER HEATING 5a: 0.5 CREDITS

ALL SHOWERHEAD and KITCHEN SINK FAUCETS INSTALLED IN THE HOUSE SHALL BE RATED AT 1.75 GPM or LESS.

ALL OTHER LAVATORY FAUCETS SHALL BE RATED AT 1.0 GPM or LESS.

EFFICIENT WATER HEATING 5c: 1.5 CREDITS

WATER HEATING SYSTEM SHALL BE:
GAS WATER HEATER WITH A MINIMUM EF OF 0.91

per w/M1503.4

MHOLE HOUSE VENTILATION

USING LAUNRDY ROOM EXHAUST FAN INTEGRATED INTO FORCED ARE SYSTEM (FAU.) PROVIDE OUTDOOR FRESH AIR W/DUCTS CONNECTED TO THE RETURN SIDE OF THE AIR HANDLER.

SYMBOL LOCATION MIN. FAN REQUIREMENTS (ALL FANS VENT TO OUTDIDE)

PROVIDE WHOLE HOUSE VENTILIATION per 2015 IRC. MI507 and IMC R403.8

BATH & Min. 50cfm. INTERMITTENT at .025wg per TABLE MI507.4

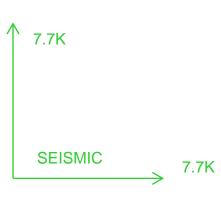
| KITCHEN | Min. 100cfm. INTERMITTENT at .025wg per TBL. MI507.4
| RANGE HOOD or DOWN DRAFT EXHAUST FAN RATED at min. 100cfm. at 0.10wg MAY BE USED FOR EXHAUST FAN REQMT. EXHAUST HOODS IN EXCESS OF 400cfm. SHALL BE INTERLOCKED AND PROVIDE MAKE UP AIR

C. LAUNDRY MIN. 180cfm. INTERMITTENT at .025mg TO FUNCTION
180cfm+ ROOM AS WHOLE HOUSE FAN (WHF.)
WHF.

MECHANICAL CONTRACTOR TO SIZE WHF. FAN and SET OPERATING TIMER PER
TABLE MI507.3.3(1) FOR A 3,001-4,500sf. DWELLING w/4-5 BEDRMS. TO OPERATE

INTERMITTENTLY and CONITINUOSLY per TABLE MI507.3.3(2)
PROVIDE CONTROLS FOR WHF. per MI507.3.2 AFFIX LABEL TO CONTROLS THAT READS "WHOLE HOUSE VENTILATION - SEE OPERATING INSTRUCTIONS"





P3 UNIT SHEAR: 630PLF/336PLF=1.88 89PLF*1.88 =167PLF 106PLF*1.88=199PLF

P3 UNIT SHEAR: 451PLF/239PLF=1.88 89PLF*1.88=167PLF 106PLF*1.88=199PLF

 SQUARE FOOTAGE SUMMARY

 MAIN FLOOR AREA + GARAGE
 2,255 S.F.

 UPPER FLOOR AREA
 1,794 S.F.

 TOTAL AREA
 4,049 S.F.

 COV'D PATIO
 235 S.F.

 COV'D PORCH
 40 S.F.

 TOTAL AREA UNDER ROOF
 4,324 S.F.

 OVERALL WIDTH
 71'-11 ½"

 OVERALL DEPTH
 44'-1 1/2"

 Updated: 03/09/2018

Method for Calculating Square Footage - ANSI Z765-2013 except: no separate distinction of 'above-grade or helphi-grade' areas and each level is measured to the

JAYMARC H O M E S

> 7525 SE 24th St., 487 Mercer Island, WA 98040 425.266.9100

. .

6515 SE 30th St. lercer Island, WA

plan name: –
marketing name: -plan number: -mark sys. number:--

Conditions not specifically represented graphically or in writing or which conflict with the current International Residential Code (IRC.) or those of the local municipality then the current standards and requirements of each respectively shall govern.

The drawings in this set are instruments of service and shall remain the property of JayMarc Homes, LLC.

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04.l5.2l Submittal Date

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Sheet Title/Description

. JAYMARC HOMES

Design Firm R.R.

Drawn by:

R.R./ S.K.

Checked by:

Primary Scale





6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD
DATE: 26-MAR-21

SHEARWALL 201: 2ND - SIDE EXTERIOR BONUS
SHEARWALL PROPERTIES:
WALL HEIGHT, H 11.5 FT. MAX WALL OPENING HT, H ₀ 3.5 FT. WALL LENGTH, L 14.4 FT. QUALIFYING WALL LENGTH, L 9.3 FT. SHEARWALL ASSEMBLY P3
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 2850 LBS 4172 LBS
SHEARWALL ASSEMBLY SPECIFICATION P3 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
Overturning Evaluation:
RESISTIVE DL 135 PLF OVERTURNING MOMENT 32.8 K-FT HOLD DOWN DESIGN LOAD 1672 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 8.7 K-FT HOLDOWN CAPACITY 1705 LBS
Hold-down Specification
SIMPSON CS16 STRAP TIE (14" END LENGTH)
SHEARWALL 202: 2ND - SIDE EXTERIOR OWNERS SUITE @ DECK
SHEARWALL PROPERTIES:
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 1.0 FT. WALL LENGTH, L 14.0 FT. QUALIFYING WALL LENGTH, L 6.0 FT. SHEARWALL ASSEMBLY P3
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 2250 LBS 2706 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P3 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION:
RESISTIVE DL 270 PLF OVERTURNING MOMENT 20.3 K-FT HOLD DOWN DESIGN LOAD 584 LBS DL AT ENDS OF WALL 70 LBS RESISTIVE MOMENT 12.1 K-FT HOLDOWN CAPACITY 1705 LBS
Hold-down Specification
SIMPSON CS16 STRAP TIE (14" END LENGTH)
SIMPSON CSTB STRAP HE (14 END LENGTH)



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

Date: 26-Mar-21

SHEARWALL 203: 2ND - SIDE EXTERIOR OWNERS SUITE
SHEARWALL PROPERTIES:
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, H ₀ 0.0 FT. WALL LENGTH, L 6.3 FT. QUALIFYING WALL LENGTH, L 6.3 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 1200 LBS 1494 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB fastened w/ 8d nails at 6"d.c. panel edges & 12"d.c. panel field - edges blocked <u>adequate</u>
OVERTURNING EVALUATION: RESISTIVE DL 270 PLF OVERTURNING MOMENT 10.8 K-FT UPLIFT CONNECTOR DESIGN LOAD 1262 LBS DL AT ENDS OF WALL 215 LBS RESISTIVE MOMENT 2.9 K-FT HOLDOWN CAPACITY 1705 LBS
HOLD-DOWN SPECIFICATION
SIMPSON CS16 STRAP TIE (14" END LENGTH)
SHEARWALL 204: 2ND - SIDE INTERIOR/EXTERIOR BATH 3
SHEARWALL 204: 2ND - SIDE INTERIOR/EXTERIOR BATH 3 SHEARWALL PROPERTIES:
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT.
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L S.6 FT. MAX WALL OPENING HT, HC WALL LENGTH, L S.6 FT. QUALIFYING WALL LENGTH, L SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL SOO LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 SB FASTENED W/ 8D NAILS AT 6 C.C. PANEL EDGES & 12 C.C. PANEL FIELD - EDGES BLOCKED
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5.0 SHEARWALL CAPACITY 5.0 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 OSB
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L S.6 FT. MAX WALL OPENING HT, HC WALL LENGTH, L S.6 FT. QUALIFYING WALL LENGTH, L SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL SOO LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 SB FASTENED W/ 8D NAILS AT 6 C.C. PANEL EDGES & 12 C.C. PANEL FIELD - EDGES BLOCKED
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L S.6 FT. MAX WALL LENGTH, L S.6 FT. MAX WALL LENGTH, L S.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 202 PLF OVERTURNING MOMENT 4.5 K-FT HOLD DOWN DESIGN LOAD 379 LES
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 500 LBS < 1338 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"0.C. PANEL EDGES & 12"0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 202 PLF OVERTURNING MOMENT 4.5 K-FT HOLD DOWN DESIGN LOAD 379 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 2.4 K-FT HOLD DOWN CAPACITY 1705 LBS
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L S.6 FT. MAX WALL OPENING HT, HC WALL LENGTH, L S.6 FT. QUALIFYING WALL LENGTH, L S.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL SDD LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1 - SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL DL AT ENDS OF WALL HOLDOWN CAPACITY 1705 LBS HOLD-DOWN SPECIFICATION
SHEARWALL PROPERTIES: WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 5.6 FT. QUALIFYING WALL LENGTH, L 5.6 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 500 LBS < 1338 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"0.C. PANEL EDGES & 12"0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 202 PLF OVERTURNING MOMENT 4.5 K-FT HOLD DOWN DESIGN LOAD 379 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 2.4 K-FT HOLD DOWN CAPACITY 1705 LBS



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD
DATE: 26-MAR-21

SHEARWALL 205: 2ND - SIDE INTERIOR/EXTERIOR BED 4		
SHEARWALL PROPERTIES:		
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, H ₀ 0.0 FT. WALL LENGTH, L 8.5 FT. QUALIFYING WALL LENGTH, L 8.5 FT. SHEARWALL ASSEMBLY P1		
CAPACITY EVALUATION:		
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 900 LBS 2032 LBS		
SHEARWALL ASSEMBLY SPECIFICATION		
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE		
OVERTURNING EVALUATION: RESISTIVE DL 202 PLF OVERTURNING MOMENT 8.1 K-FT UPLIFT CONNECTOR DESIGN LOAD 544 LBS		
DL AT ENDS OF WALL 70 LBS RESISTIVE MOMENT 3.5 K-FT HOLDOWN CAPACITY 1705 LBS		
HOLD-DOWN SPECIFICATION		
SIMPSON CS16 STRAP TIE (14" END LENGTH)		
SHEARWALL XXX: - NOT USED		
SHEARWALL XXX: - NOT USED SHEARWALL PROPERTIES:		
SHEARWALL PROPERTIES: WALL HEIGHT, H		
SHEARWALL PROPERTIES: WALL HEIGHT, H		
SHEARWALL PROPERTIES: WALL HEIGHT, H		
SHEARWALL PROPERTIES: WALL HEIGHT, H		
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L O.O FT. MAX WALL OPENING HT, HC O.O FT. SHEARWALL ASSEMBLY PO CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY BHEARWALL ASSEMBLY SPECIFICATION PO - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O!		
SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L WALL LEN		
SHEARWALL PROPERTIES: WALL HEIGHT, H		
SHEARWALL PROPERTIES: WALL HEIGHT, H		



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 206: 2ND - FRONT EXTERIOR BED2/BATH3			
SHEARWALL PROPERTIES:			
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, H 6.0 FT. WALL LENGTH, L 20.0 FT. QUALIFYING WALL LENGTH, L 17.5 FT. SHEARWALL ASSEMBLY P1			
CAPACITY EVALUATION:			
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 2650 LBS 4123 LBS			
SHEARWALL ASSEMBLY SPECIFICATION			
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE			
OVERTURNING EVALUATION: RESISTIVE DL 350 PLF OVERTURNING MOMENT 23.9 K-FT UPLIFT CONNECTOR DESIGN LOAD 0 LBS DL AT ENDS OF WALL 200 LBS RESISTIVE MOMENT 32.6 K-FT HOLDOWN CAPACITY 0 LBS			
HOLD-DOWN SPECIFICATION			
No Holdown Required			
SHEARWALL 207: 2ND - REAR EXTERIOR OWNERS SUITE SHEARWALL PROPERTIES: WALL HEIGHT, H 12.0 FT. MAX WALL OPENING HT, HC 0.0 FT. WALL LENGTH, L 9.8 FT. QUALIFYING WALL LENGTH, L 9.8 FT. SHEARWALL ASSEMBLY P1			
GAPAGITY EVALUATION:			
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 1250 LBS 2330 LBS			
SHEARWALL ASSEMBLY SPECIFICATION			
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE			
OVERTURNING EVALUATION: RESISTIVE DL 135 PLF OVERTURNING MOMENT 15.0 K-FT HOLD DOWN DESIGN LOAD 1218 LBS DL AT ENDS OF WALL 70 LBS RESISTIVE MOMENT 3.1 K-FT HOLDOWN CAPACITY 1705 LBS			
HOLD-DOWN SPECIFICATION			
SIMPSON CS16 STRAP TIE (14" END LENGTH)			



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 208: 2ND - REAR EXTERIOR LND/WIC
SHEARWALL PROPERTIES:
WALL HEIGHT, H 9.0 FT. MAX WALL OPENING HT, H _c 3.5 FT. WALL LENGTH, L 21.0 FT. QUALIFYING WALL LENGTH, L 19.0 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 3800 LBS 4541 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION:
RESISTIVE DL 245 PLF OVERTURNING MOMENT 34.2 K-FT UPLIFT CONNECTOR DESIGN LOAD 321 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 27.5 K-FT HOLDOWN CAPACITY 1705 LBS
HOLD-DOWN SPECIFICATION
SIMPSON CS16 STRAP TIE (14" END LENGTH)
SHEARWALL XXX: - NOT USED
SHEARWALL PROPERTIES:
WALL HEIGHT, H
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY LBS #### #DIV/O! LBS
SHEARWALL ASSEMBLY SPECIFICATION
PO - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O!
#514/51
OVERTURNING EVALUATION:
RESISTIVE DL 0 PLF OVERTURNING MOMENT #DIV/O! K-FT HOLD DOWN DESIGN LOAD 0 LBS
DL AT ENDS OF WALL OLS RESISTIVE MOMENT O.O K-FT HOLDOWN CAPACITY OLS
HOLD-DOWN SPECIFICATION
No Holdown Required



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL PROPERTIES:
WALL HEIGHT, H 11.0 FT. MAX WALL OPENING HT, H WALL LENGTH, L 21.2 FT. QUALIFYING WALL LENGTH, L 21.2 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 600 LBS 5067 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION:
RESISTIVE DL 135 PLF OVERTURNING MOMENT 6.6 K-FT UPLIFT CONNECTOR DESIGN LOAD 0 LBS DL AT ENDS OF WALL 800 LBS RESISTIVE MOMENT 20.8 K-FT HOLDOWN CAPACITY 0 LBS
HOLD-DOWN SPECIFICATION
No Holdown Required
SHEARWALL 102: 1st - Side Interior Garage
SHEARWALL PROPERTIES:
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 0.0 FT.
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: Total Shear Load on Wall Allowable Shearwall Capacity
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE.
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION:
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" OSB FASTENED W/ BD NAILS AT 6"0.C. PANEL EDGES & 12"0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 315 PLF OVERTURNING MOMENT 62.3 K-FT HOLD DOWN DESIGN LOAD 1197 LBS
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L 21.7 FT. SHEARWALL ASSEMBLY P1 CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16 SB FASTENED W/ BD NAILS AT 6 0.C. PANEL EDGES & 12 0.C. PANEL FIELD - EDGES BLOCKED ADEQUATE OVERTURNING EVALUATION: RESISTIVE DL 315 PLF OVERTURNING MOMENT 62.3 K-FT HOLD DOWN DESIGN LOAD 1197 LBS DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 36.4 K-FT HOLD DOWN CAPACITY 3695 LBS
WALL LENGTH, L 21.7 FT. QUALIFYING WALL LENGTH, L CAPACITY EVALUATION: TOTAL SHEAR LOAD ON WALL 5050 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE DVERTURNING EVALUATION: RESISTIVE DL 315 PLF OVERTURNING MOMENT ACLOWABLE SHEARWALL CAPACITY 5070 LBS SHEARWALL ASSEMBLY SPECIFICATION P1 - 1-SIDE 7/16" DSB FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE DVERTURNING EVALUATION: RESISTIVE DL 315 PLF OVERTURNING MOMENT 36.4 K-FT HOLD DOWN DESIGN LOAD 1197 LBS HOLD-DOWN SPECIFICATION



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL 103: 1ST - SIDE INTERIOR GREAT RM	
SHEARWALL PROPERTIES:	
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, H $_{c}$ 0.0 FT.	
WALL LENGTH, L 13.0 FT. QUALIFYING WALL LENGTH, L 13.0 FT. SHEARWALL ASSEMBLY P1	
CAPAGITY EVALUATION:	
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY	
2250 LBS < 3107 LBS	
SHEARWALL ASSEMBLY SPECIFICATION	
P1 - 1-SIDE 7/16" OSB	
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED	
<u>ADEQUATE</u>	
Overturning Evaluation:	
	BS
DL AT ENDS OF WALL 800 LBS RESISTIVE MOMENT 24.5 K-FT HOLDOWN CAPACITY 1705	.BS
HOLD-DOWN SPECIFICATION	
SIMPSON CS16 STRAP TIE (14" END LENGTH)	
SHEARWALL 104: 1st - Side Exterior Kitchen/Scullery	
SHEARWALL PROPERTIES:	
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, HC 7.0 FT. WALL LENGTH, L 18.0 FT. QUALIFYING WALL LENGTH, L 15.5 FT. SHEARWALL ASSEMBLY P1	
CAPACITY EVALUATION:	
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 1800 LBS 3675 LBS	
LBS C 3075 LBS	
SHEARWALL ASSEMBLY SPECIFICATION	
P1 - 1-SIDE 7/16" OSB	
FASTENED W/ 8D NAILS AT 6"D.C. PANEL EDGES & 12"D.C. PANEL FIELD - EDGES BLOCKED ADEQUATE	
Overturning Evaluation:	
RESISTIVE DL 505 PLF OVERTURNING MOMENT 18.0 K-FT HOLD DOWN DESIGN LOAD 0	.BS
DL AT ENDS OF WALL 470 LBS RESISTIVE MOMENT 39.7 K-FT HOLDOWN CAPACITY 0	.BS
HOLD-DOWN SPECIFICATION	
No Holdown Required	
NO FIDEDOWN REQUIRED	
<u></u>	



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD
DATE: 26-MAR-21

SHEARWALL XXX: - NOT USED
SHEARWALL PROPERTIES:
WALL HEIGHT, H O.O FT. MAX WALL OPENING HT, H _c O.O FT. WALL LENGTH, L O.O FT. QUALIFYING WALL LENGTH, L O.O FT. SHEARWALL ASSEMBLY PO
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY LBS #### #DIV/O! LBS
SHEARWALL ASSEMBLY SPECIFICATION
PO - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED #DIV/O!
OVERTURNING EVALUATION: RESISTIVE DL
Hold-down Specification
No Holdown Required
SHEARWALL 105: 1ST - FRONT EXTERIOR GREAT RM @ DINING SHEARWALL PROPERTIES: WALL HEIGHT, H WALL LENGTH, L 7.7 FT. QUALIFYING WALL LENGTH, L 5.3 FT. SHEARWALL ASSEMBLY P3
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL 800 LBS ALLOWABLE SHEARWALL CAPACITY 1942 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P3 - 1-SIDE 7/16" OSB FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE
OVERTURNING EVALUATION: RESISTIVE DL 480 PLF OVERTURNING MOMENT 10.5 K-FT HOLD DOWN DESIGN LOAD 202 LBS DL AT ENDS OF WALL 800 LBS RESISTIVE MOMENT 8.9 K-FT HOLDOWN CAPACITY 3695 LBS
Hold-down Specification
SIMPSON STHD14RJ HOLDOWN



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SHEARWALL IUG: 1ST - FRONT EXTERIO GREAT RM
SHEARWALL PROPERTIES:
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, H_c 7.0 FT.
WALL LENGTH, L 6.3 FT. QUALIFYING WALL LENGTH, L 4.0 FT. SHEARWALL ASSEMBLY P3
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 650 LBS 1466 LBS
LBS LBS
SHEARWALL ASSEMBLY SPECIFICATION
P3 - 1-SIDE 7/16 OSB
FASTENED W/ 8D NAILS AT 3 ^o c.c. panel edges & 12 ^o c.c. panel field - edges blocked ADEQUATE
APERIONIE.
OVERTURNING EVALUATION:
RESISTIVE DL 480 PLF OVERTURNING MOMENT 8.5 K-FT UPLIFT CONNECTOR DESIGN LOAD 335 LBS
DL AT ENDS OF WALL 800 LBS RESISTIVE MOMENT 6.4 K-FT HOLDOWN CAPACITY 3695 LBS
HOLD-DOWN SPECIFICATION
SIMPSON STHD14RJ HOLDOWN
SHEARWALL 107: 1st - Rear Interior Garage
SHEARWALL IDT. IST REAR INTERIOR GARAGE
Shearwall Properties:
WALL HEIGHT, H 11.0 FT. MAX WALL OPENING HT, HC 0.0 FT.
WALL LENGTH, L 17.0 FT. QUALIFYING WALL LENGTH, L 17.0 FT. SHEARWALL ASSEMBLY P1
CAPACITY EVALUATION:
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY 3700 LBS 4063 LBS
3700 LBS 4063 LBS
SHEARWALL ASSEMBLY SPECIFICATION
P1 - 1-SIDE 7/16" OSB
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED
<u>ADEQUATE</u>
OVERTURNING EVALUATION:
RESISTIVE DL 440 PLF OVERTURNING MOMENT 40.7 K-FT HOLD DOWN DESIGN LOAD 573 LBS
DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 31.0 K-FT HOLDOWN CAPACITY 3695 LBS
HOLD-DOWN SPECIFICATION
SIMPSON STHD14RJ HOLDOWN



6515 SE 30TH ST

M&K PROJECT #: 154-21007

ENGINEER: RJD
DATE: 26-MAR-21

SHEARWALL 108: 1st - Front Exterior Garage					
SHEARWALL PROPERTIES:					
WALL HEIGHT, H 10.0 FT. MAX WALL OPENING HT, H _c 8.0 FT.					
WALL LENGTH, L 21.6 FT. QUALIFYING WALL LENGTH, L 5.5 FT. SHEARWALL ASSEMBLY P3					
CAPACITY EVALUATION:					
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY					
2100 LBS < 2108 LBS					
SHEARWALL ASSEMBLY SPECIFICATION					
P3 - 1-SIDE 7/16" OSB					
FASTENED W/ 8D NAILS AT 3"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - EDGES BLOCKED ADEQUATE					
OVERTURNING EVALUATION:					
RESISTIVE DL 280 PLF OVERTURNING MOMENT 21.0 K-FT UPLIFT CONNECTOR DESIGN LOAD 0 LBS					
DL AT ENDS OF WALL 400 LBS RESISTIVE MOMENT 32.5 K-FT HOLDOWN CAPACITY 0 LBS					
HOLD-DOWN SPECIFICATION					
No Holdown Required					
TO THE SECTION OF THE					
SHEARWALL XXX: - NOT USED					
CHEADWALL BRODERTIES:					
SHEARWALL PROPERTIES:					
WALL HEIGHT, H O.O FT. MAX WALL OPENING HT, HC O.O FT. WALL LENGTH, L O.O FT. QUALIFYING WALL LENGTH, L O.O FT. SHEARWALL ASSEMBLY PO					
CAPACITY EVALUATION:					
TOTAL SHEAR LOAD ON WALL ALLOWABLE SHEARWALL CAPACITY O LBS #### #DIV/O! LBS					
SHEARWALL ASSEMBLY SPECIFICATION					
PO - 1-SIDE 7/16" OSB					
FASTENED W/ 8D NAILS AT 6"O.C. PANEL EDGES & 12"O.C. PANEL FIELD - UNBLOCKED					
#DIV/0!					
OVERTURNING EVALUATION					
OVERTURNING EVALUATION: RESISTIVE DL					
DL AT ENDS OF WALL O LBS RESISTIVE MOMENT O.O K-FT HOLDOWN CAPACITY O LBS					
Hold-down Specification					
No Holdown Required					



PROJECT NAME: 6515 SE 30TH - DECK

JAYMARC HOMES

M&K PROJECT #: 154-21007

ENGINEER: RJD

DATE: 26-MAR-21

SEISMIC CALCULATION - ASCE 7-16

1					
STEP CLASS D	SEISMIC DESIGN CAT	EGORY:		BUILDING PERIOD DETERM	INATION:
SPECITIAL RESPONDE AGGLERATION 1.0 SEG, 81 1.0016 PROBLEM TRANS PRESEND, T, (see) 5 5 1.0016 PROBLEM TRANS PRESEND, T, (see) 5 5 1.0016 PROBLEM TRANS PRESEND, T, (see) 5 5 5 5 5 5 5 5 5	USER INPUTS:			USER INPUTS:	
REPOTRIAL REPORTER ARCHITECTURE, No. 10.0000 Proceedings	SITE CLASS		D	BUILDING PERIOD COEFFICIENT,	C _T 0.020
REPOTRIAL REPORTER ARCHITECTURE, No. 10.0000 Proceedings	SPECTRAL RESPO	INSE ACCELERATION 0.2 SEC. 88	1 405	LONG-PERIOD TRANS PERIOD T	(SEC) 6
DECUMPANCY CATEGORY					
### SHIP CONTROLLED FA 1.20 T. 0.007 ### CONTROLLE	SPECTRAL RESPO	INSE ACCELERATION 1.0 SEC, S1	0.489	HT. ABV BASE TO HIGHEST LEVE	:∟, h _N 5
VARIABLEST STITE PROPERTION F.	OCCUPANCY CAT	EGORY	11	CALCULATED VALUES:	
SITE DOLFFIDIENT, FA	VARIARI ES:			· · · · · · · · · · · · · · · · · · ·	DD T 0.067
STIC COSTRICATION TO					
MARINIA PETERAL REPRINE ASSISTANT Section 1.0810	SITE COEFFICIENT	', FA	1.20	To	0.105
MAXIMUM DESTINAL RESPONSE ADDICATION Substitution Substituti	SITE COEFFICIENT	, FV	1.81	Ts	0.525
MAXIMUM SECTIMAL REPORTE ACCELERATION, 80, DIRECTION SECTION, 80, DIRECTION SECTION, 80, DIRECTION,	CALCULATED VALL	IES:			
Defend operation appropriate Acceleration Signature Defend operation Defends appropriate Defends app				SPECTRAL RESPONSE ACC., S _A (G)	0.879
Debind Spectral Response Additional Property In Section 11.6.3 The Property Prop					
Decision Septemble Addressarion, Box D.590 To get	MAXIMUM SPECTE	AL RESPONSE ACCELERATION, S _M 1	0.886		
Design Price of Reference Acceleration Bay Design Price of Reference Color Tend Design Category (1.0 second remote Design Category (1.0 second re	DESIGN SPECTRAL RESPONSE ACCELERATION, Spe		1.124	SITE CLASS ASSUMPTION	
SERMO DERIGNO CATEGORY (SHORT FERM) D			0.590	YES PER ASCE 7-	16 SECTION 11.4.3
RESIDENCE DESIGNATE FOR PRODUCTION Dead Local Cost					SS MAY BE ASSUMED
PRAD LOAD CALCULATION:				TO BE D	
DEAD LOAD CALCULATION:	SEISMIC DESIGN	CATEGORY (T.O SECOND TERM)	D		
	EQUIVALENT LATE	RAL FORCE PROCEDURE	<u> </u>		
	DEAD LOAD CALC	ULATION:			
				DL OF EXT WALL TRIB.	
1	I FVFI	STORY HT. (FT.) AREA (FT) DEAD LOAD (PRF)		TOTAL LEVEL D
2 0.0 0 0 0 0.0 0 0.0 0 0.0 0 0.0 0.0 0 0	LEVEL				
3					
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5 0.0 0 0 0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	=				
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7 0.0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0					
B					
9					
10					
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12					
13					
1.4					
15					
16					
17					
18					
19					
TOTAL DEAD LOAD OF STRUCTURE 2					
SEISMIC RESPONSE COEFFICIENT: TRANSVERSE LONGITUDINAL 6.5 6.5					
SEISMIG RESPONSE COEFFICIENT: TRANSVERSE LONGITUDINAL					
TRANSVERSE LONGITUDINAL LONGIT				TOTAL DEAD LOAD OF STR	UCTURE 2
RESPONSE MODIFICATION FACTOR, R OCCUPANCY IMPORTANCE FACTOR, Ig SEISMIC RESPONSE COEFFICIENT, Og OLT73 OLT73 D.173 D.173 TRANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, Og ULTIMATE LOADS ** O	<u>SEISMIC RESPONS</u>				
Science Response Coefficient Coefficie	RESPONSE MODIFI	SATION FACTOR, R 6.5	6.5		
BASE SHEARS:	OCCUPANCY IMPOR	TANCE FACTOR, IE 1.00	1.00		
BASE SHEARS:	S B		0.172		
TRANSVERSE LONGITUDINAL TRANSVERSE LONGITUDINAL O K 0.3 K 0.	SEISMIC RESPONS	E GOEFFICIENT, C. 0.173	0.173		
D K D K D K D K D K D K D K D B B B B B B B B B	BASE SHEARS:	ULTIMATE LOADS	x 0.7 =	ALLOWABLE LOADS	
STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00		TRANSVERSE LONGITUDIN	.AL	TRANSVERSE	LONGITUDINAL
STORY SHEAR CALCULATION: DISTRIBUTION EXPONENT, 1.00			7 ,	0.3	пз
DISTRIBUTION EXPONENT,			_ `		5.3
Name	STORY SHEAR CAL	.CULATION:			
Nert. Dist. Transverse Story Shear, F. S	DISTRIBUTION EXPONE	√T, 1 1.00			
Transverse Constitutional Co		ULTIMATE LOADS	x D.7 =	ALLOWABLE LOADS	
EVEL EATIDR. 1.000	VERT. DIST.	TRANSVERSE LONGITUDIN			
2 0.000 0.0 K 0.0		STORY SHEAR, F . STORY SHEAR	STORY 5	SHEAR, F_* Σ STORY SHEAR STORY SHE	EAR, F _* Σ STORY SHEA
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3 0.000 0.0 K 0.0			_		
5 0.00 0.0 K			_		
6			к 🔲	к к	к 0.0
7 0.00 0.0 K	3 0.000	0.0 K 0.0			к 0.0
8 0.00 0.0 K	3 0.000				
9 0.00 0.0 K	3 0.000 4 0.000 5 0.00 6 0.00	0.0 K 0.0	к	1.0 к 0.0 к 0.0	
10	3	0.0 K 0.0 C O.0 K 0.0	к 🗆	0.0 K 0.0 K 0.0	к 0.0
11 0.00 0.0 K 0.0	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00	0.0 K 0.0 0.0 K 0.0 0.0 K 0.0	к	1.0 K 0.0 K 0.0 1.0 K 0.0 K 0.0	к 0.0
12	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00	0.0 K 0.0 0.0 K 0.0 0.0 K 0.0 0.0 K 0.0	к	1.0 K 0.0 K 0.0 1.0 K 0.0 K 0.0 1.0 K 0.0 K 0.0	K 0.0 K 0.0
13 0.00 0.0 K 0.0	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00	0.0 K 0.0	K	1.0 K 0.0 K 0.0 1.0 K 0.0 K 0.0 1.0 K 0.0 K 0.0 1.0 K 0.0 K 0.0	к 0.0 к 0.0 к 0.0
14 0.00 0.0 K 0.0	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00 10 0.00	0.0 K 0.0	K	1.0 K 0.0 K 0.0	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0
15	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00 10 0.00 11 0.00 12 0.00	0.0 K 0.0	K	1.0 K 0.0 K 0.0	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0
16 0.00 0.0 K 0.0	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00 10 0.00 11 0.00 12 0.00 13 0.00	0.0 K 0.0	K	1.0 K 0.0 K 0.0	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0
17 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 18 0.0 K 0.0 K 0.0 19 0.00 K 0.0 K 0	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00 10 0.00 11 0.00 12 0.00 13 0.00	0.0 K 0.0	K	1.0 K 0.0 K 0.0	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0
18 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 C K 0.0 C C C C C C C C C C C C C C C C C C	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00 10 0.00 11 0.00 12 0.00 13 0.00 14 0.00	0.0 K 0.0	K	1.0 K 0.0 K 0.0	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0
19 0.00 0.0 K 0.0 K 0.0 K 0.0 K 0.0	3	0.0 K 0.0	K	1.0 K 0.0 K 0.0	K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0 K 0.0
	3	0.0 K 0.0	K	1.0	K 0.0 K 0.0
	3 0.000 4 0.000 5 0.00 6 0.00 7 0.00 8 0.00 9 0.00 10 0.00 11 0.00 12 0.00 13 0.00 14 0.00 15 0.00 16 0.00 17 0.00	0.0 K 0.0	K	1.0	K 0.0 K 0.0
	3	0.0 K 0.0	K	1.0	K 0.0 K 0.0

DATE: (14-13-21

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